

chartered consulting engineers

Our ref: 8007\_FCA

# Flood Consequences Assessment and Drainage Strategy

for

Midnant Farm, Gronant Road

Prestatyn

Denbighshire

For: Castle Green Homes Ltd

Unit 20, St Asaph Business Park

St Asaph Denbighshire LL17 0LJ

#### **Document Verification**

Project Title	Midnant Farm, Gronant Road, Prestatyn, Denbighshire
Project Number	8007
Document Title	Flood Consequences Assessment and Drainage Strategy
<b>Document Number</b>	8007_FCA_Issue 1

This document is not to be used for contractual or engineering purposes unless the document verification sheet is signed where indicated by the approver of the document.

Prepared by Checked and Approved

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Peter L. Syles

#### **Document Revision**

Report	Date	Description	Prepared	d Checked and	
Reference				Approved	
8007_FCA	25/11//2022	Flood Consequences	A Jones	P R Sykes	
Issue 1		Assessment	A Julies	r K Sykes	

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#### 1.0 Introduction

Coopers (Chester) Ltd, (Coopers) have been appointed by Castle Green Homes Ltd to assess the risk of flooding and to provide a Drainage Strategy for Midnant Farm located off Gronant Road, Presatyn. Castle Green Homes Ltd are proposing a new housing development, comprising of approximately 45 No. dwellings.

Castle Green Homes Ltd are planning the construction of a mixture of semi-detached and detached residential properties with associated access road, parking, vehicular access and landscaping subject to conditions. It is understood the site does not currently benefit from any planning decision.

This flood consequences assessment (FCA) evaluates the proposals with regard to flood risk, identifying and appraising potential flood risk both to and from the whole site. Coopers have carried out the following:

- Assessment of the development potential of the site in line with the Welsh Government's Technical Advice Note 15: Development and Flood Risk (TAN15) and;
- ii. An assessment of surface water runoff and drainage strategy

Since January 7th, 2019, all new developments will require sustainable drainage for surface water if there are at least 2 No. properties or the construction area is more than 100m<sup>2</sup>. The surface water drainage systems must be designed and built to meet Welsh Government standards for sustainable drainage.

These systems must be approved by the local authority acting in its SUDS Approving Body (SAB) role before construction work begins. The SAB will have a duty to adopt compliant systems.

#### 2.0 Site Characteristics

#### 2.1 Site Location

The site is a parcel of agricultural land in Prestatyn. The site is situated off Gronant Road at approximate grid reference SJ077831.

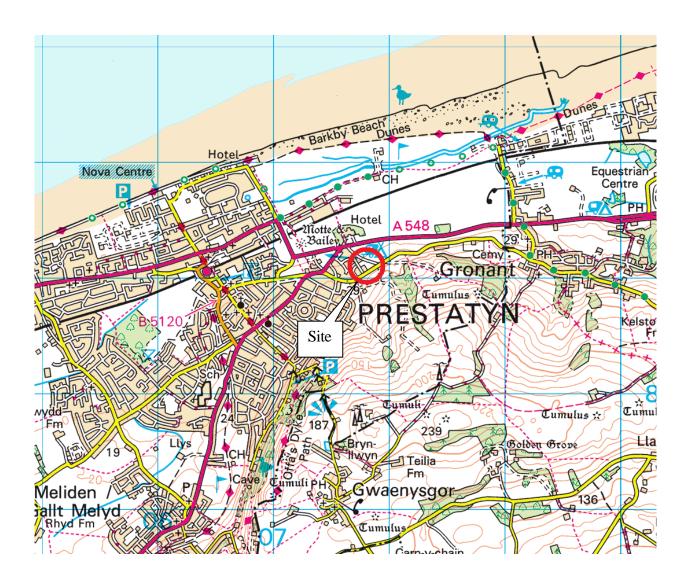


Figure 1 – Site Location

#### 2.2 Site Description

The site covers an area of 1.45 Hectares and consists of a farmhouse with associated barns, outbuildings, and hard standings.

The topography of site falls in a northerly directly and levels vary from a highpoint of 29.8m AOD at the southern end of the side along Gronant Road and a low point of 15.5m AOD to the northern end of the site. Refer to topographical survey in Appendix 1.

#### 3.0 Sources of Flood Risk Information

#### 3.1 The Welsh Government Development Advice Map

The Welsh Government Development Advice Map shows the site is located within Flood Zone A – an area considered to be at little or no risk of fluvial or tidal flooding, with a less than 1 in 1000 (0.1%) annual probability of flooding in any given year.

The proposed residential development is considered to be 'highly vulnerable' development in accordance with Figure 2 of the Welsh Governments Technical Advice Note 15. Highly vulnerable development is considered to be appropriate within Flood Zone A.

#### 3.2 Natural Resources Wales

The NRW Flood Map shows the site is located within Flood Zone 1 – an area considered to have the lowest probability of fluvial flooding. It is assessed as having a less than 0.1% annual probability of flooding in any given year.

It should be noted that the Flood Map only covers flooding from rivers and the sea. Flooding can occur at any time and in any place from sources such as rising groundwater levels, burst water mains, blocked road drains, run-off from hillsides, sewer overflows, etc.



Figure 2 – Natural Resources Wales Flood Map for Planning (Rivers)

The site is located approximately 1.2km from the coastline. The nearest major watercourse is the Prestatyn Gutter which is located approximately 730m north of the site. The Prestatyn Gutter flows north-east in this location to its discharge to the sea. Other watercourses in the area include an unnamed drain located approximately 320m northeast of the site.



Figure 3 – Natural Resources Wales Flood Map for Planning (Sea)

The Natural Resources Wales long term flood risk maps indicate the site has a low risk of flooding from Surface Water.



Figure 4 – Natural Resources Wales Surface Water Flooding Map

#### 3.3 <u>Denbighshire LLFA</u>

The Denbighshire Council Local Flood Risk Management Strategy (June 2014) contains no records of any flooding at or near to the site. We have contacted Denbighshire Council for confirmation of any known historical flooding within the vicinity of the site and are currently waiting for a response.

### 4.0 Sources of Flood Risk

#### 4.1 Fluvial

Extreme fluvial flood events have the potential to cause rapid inundation of the site whilst posing a threat to welfare and users. As outlined in Section 3.2; the site is within Flood Zone 1 and is therefore not at risk from extreme fluvial or tidal flooding. Therefore, the risk from extreme fluvial flooding to the site is considered to be low.

#### 4.2 <u>Infrastructure Failure (Existing and Proposed)</u>

The failure of infrastructure such as culverts or bridges could increase the risk of flooding at the site. The risk of flooding is considered as very low.

#### 4.3 Overland Flow

Overland flow occurs when the infiltration capacity of the ground is exceeded in a storm event. This can result in water travelling as a sheet flow overland or excess water being conveyed from location to another via local road networks. Due to the topography of the site sloping to the northern end of the site overland flow from within the site is not considered a significant risk. Overland flows from the site will be significantly reduced post development with the incorporation of positive drainage and an internal road network.

We have reviewed the surrounding topography and drainage infrastructure for the surrounding area and note there is potential for overland flows entering the site from higher ground from the south. This includes Gronant Road which does not appear to have any obvious highway drainage.

Further investigation on Gronant Road existing drainage should be undertaken and if necessary new drainage for Gronanat Road will need to be incorporated into the design to reduce the potential of overland flows from off-site sources post development.

#### 4.4 Sewer Flooding

If the capacity of the sewers is exceeded in an extreme event, or a blockage occurs, surcharging of the network can result in surface flooding. Welsh Water sewer plans which are included in Appendix 2, indicate that there are currently no existing adopted sewers located within the site boundary.

Welsh Water may have confirmed they have no records or any known flooding within the vicinity of the site. Refer to Appendix 5 for correspondence.

The overall risk from sewer flooding is considered as low.

#### 4.5 Groundwater Flooding

Groundwater flooding occurs as a result of water rising up from the underlying superficial deposits, bedrock or from springs. Shallow groundwater can exacerbate the impact of other sources of flooding such as pluvial flooding, increasing the likelihood of overland flow or standing water expected to occur in the naturally low-lying areas within the site

The groundwater flooding susceptibility is categorised as potential for ground water flooding on the BGS Flood Data Map but is shown to be at Negligible Risk on the GeoSmart Information Groundwater Flood Risk Map. Refer to Appendix 3 for Envirocheck Flood Data

The overall risk from groundwater flooding is considered as low.

#### 4.6 Coastal Flooding

The site is not located in proximity of any tidal waterway or within close proximity to the sea and is therefore not at risk from tidal inundation.

#### 4.7 Reservoirs

The site is not located in proximity of any reservoirs. Additionally, the NRW maps indicate the site is not at risk of flooding from reservoirs.

### 5.0 Surface Water Drainage

#### 5.1 General

The design for a surface water drainage system for the proposed development will be guided by the principles set out in the Welsh Government's 'Recommended non-statutory standards for sustainable drainage (SUDS) in Wales – designing, constructing, operating and maintaining surface water drainage systems' (2017)

The SUDS Standards Wales sets out the following hierarchy for surface water runoff destination:

Priority Level 1: Surface water runoff is collected for use;

Priority Level 2: Surface water runoff is infiltrated to ground;

Priority Level 3: Surface water runoff is discharged to a surface water body;

Priority Level 4: Surface water runoff is discharged to a surface water sewer, highway drain, or another drainage system;

Priority Level 5: Surface water runoff is discharged to a combined sewer.

Note that Priority Level 1 is the preferred (highest priority) and that 4 and 5 should only be used in exceptional circumstances.

#### 5.2 Existing Surface Water Drainage

We have not been able to confirm how the site currently disposes of surface water flows. We can confirm the existing buildings have rainwater pipes taking roof run-off into the ground via pipework but its unclear whether these discharge to soakaways or a piped network takes flows off-site. This will need to be confirmed via drainage surveys when the client has been permitted site access.

The utilities survey has identified a possible culvert crossing the site from east to west at a depth of 1.3m. This would potentially allow for gravity connections from all existing building roof run-off but will need confirming via drainage surveys. If this is found to be a culvert the off-site route and outfall should also be investigated before we can consider any possible re-use and liaise with relevant asset owners.

#### 5.3 Existing Site Runoff

The greenfield run-off rates for the site has been calculated using the HR Wallingford Greenfield runoff rate estimation tool.

QBAR = 
$$7.2 \text{ l/s}$$
 (assuming clay site – soil type 4)

Refer to Appendix 7 for calculations.

We have also calculated the existing brownfield flow rates for the site based on the existing roof measures areas. This provided much higher flow rates but will only be relevant if the drainage survey confirms all roof drainage is connected to a piped network which takes flows off-site.

Using the Modified Rational Method as described by the Wallingford Procedure:

$$Qp = C i A$$

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Qp = runoff (litres /second) C = runoff coefficient i = intensity A = Area

If the units are as follows, then the equation becomes Qp = 2.78C i A;

$$i = mm/hr$$
  
 $A = hectares$ 

Runoff coefficient C = Cv Cr, as standard values of 0.75 and 1.30 are used for Cv and Cr respectively.

$$Qp = 3.61Cv i A$$
$$= 2.71 i A$$

Existing Roof Area 'A' is measured at 0.2ha.

Rainfall intensity 'i,' has been calculated for different storm events:

1-year = 25.8 mm/hr 30-year = 63.1 mm/hr 100-year = 81.4 mm/hr

Therefore, *Qp for the different return period is as follows:* 

```
1-year = 2.71 x 25.8 x 0.20 = 13.98 l/s
30-year = 2.71 x 63.1 x 0.20 = 34.20 l/s
100-year = 2.71 x 81.4 x 0.20 = 44.12 l/s
```

#### 5.4 <u>Proposed Surface Water Drainage</u> and Runoff Rates

#### Priority Level 1

Whilst rainwater harvesting has been considered for the proposed development it should be noted that any device enabling water re-use cannot be taken into account when sizing attenuation as the storage facility may be full when a storm event occurs. Therefore, an overflow to an infiltration device (where ground conditions allow) or to a watercourse / sewer will be required.

Castle Green Ltd are not proposing to incorporate rainwater harvesting within the development; however, they are proposing to install a water butt to each dwelling which will allow for water collection for garden re-use.

#### Priority Level 2

No site investigation has been undertaken in advance of this report being written as the client has been unable to access the site. We have reviewed the available geology data which is presented in Appendix 4. There does look like the site can potentially drain via infiltration into underlying sands /

gravels, but this will need to be confirmed during intrusive site investigation and infiltration tests within the site.

Even if slow rates of infiltration are confirmed whilst this will not be good enough to drain the site during a 100-year storm event they will potentially provide for slow infiltration SUDS components to deal with low flow events and provide for 5mm interception. Permeable paving (partial infiltration) and bioretention components such as tree pits and raingardens can be considered appropriate.

#### Priority Level 3

The site is located approximately 1.2km from the coastline. The nearest major watercourse is the Prestatyn Gutter which is located approximately 730m north of the site. The Prestatyn Gutter flows north-east in this location to its discharge to the sea. Other watercourses in the area include an unnamed drain located approximately 320m northeast of the site on the northern side of Prestatyn Road

We have no level information on this watercourse, so we are unable to confirm if a gravity connection into this watercourse is achievable. This is also within third party land so consent would be required by the landowner.

#### Priority Level 4

A review of the Welsh Water sewer maps indicates an existing 150mm Dia surface water sewer in Rhodfa Celyn located to the west of the site. This flows north and discharges into a larger 225mm Dia surface water sewer at the junction of Nant Drive and Prestatyn Road.

Welsh Water may accept restricted flows into one of these assets but would first request a full evidence-based report exploring all other higher priority destinations (reuse, infiltration, watercourses) for their consideration.

For the purpose of this drainage strategy we are proposing a discharge into the 225mm Dia surface water sewer at the junction of Nant Drive and Prestatyn Road. Flows will be restricted to greenfield QBAR of 7.2 l/s. A full topographical and utilities survey will be required to ensure a gravity connection into this sewer is achievable.

#### 5.5 SUDS Approval Bodies

Since January 7th, 2019, all new developments will require sustainable drainage for surface water if there are at least 2No. properties or the construction area is more than 100m<sup>2</sup>. The surface water drainage systems must be designed and built to meet Welsh Government standards for sustainable drainage.

These systems must be approved by the local authority acting in its SUDS Approving Body (SAB) role before construction work begins. The SAB will have a duty to adopt compliant systems.

Every SUDS application should go to every attempt to satisfy the Principles and Standards of the legislation. When vetting an application, the SAB officer will look at the clear red line boundary area

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of the site when considering space for SUDS and water management features and not the space that's

left on the proposed site layout.

The principles are as follows:

SUDS schemes should aim to:

- 1. manage water on or close to the surface and as close to the source of the runoff as possible;
- 2. treat rainfall as a valuable natural resource;
- 3. ensure pollution is prevented at source, rather than relying on the drainage system to treat or intercept it;
- 4. manage rainfall to help protect people from increased flood risk, and the environment from morphological and associated ecological damage resulting from changes in flow rates, patterns and sediment movement caused by the development;
- 5. take account of likely future pressures on flood risk, the environment and water resources such as climate change and urban creep;
- 6. use the SUDS Management Train, using drainage components in series across a site to achieve a robust surface water management system (rather than using a single "end of pipe" feature, such as a pond, to serve the whole development);
- 7. *maximise the delivery of benefits for amenity and biodiversity;*
- 8. seek to make the best use of available land through multifunctional usage of public spaces and the public realm;
- 9. perform safely, reliably and effectively over the design life of the development taking into account the need for reasonable levels of maintenance;
- 10. avoid the need for pumping where possible; and
- 11. be affordable, taking into account both construction and long-term maintenance costs and the additional environmental and social benefits afforded by the system.

Applicants seeking SAB Approval must demonstrate how they have complied with these principles or provide justification for any departure.

The preference is to discharge all surface water to ground via infiltration techniques such as soakaways, permeable paving and possibly an infiltration basin. However, as no site investigation has been undertaken, we have developed a surface water 'option 2' scheme with a discharge into the surface water public sewer. An indicative infiltration layout is presented in Appendix 6 (Drg No. 8007/SK02).

The surface water strategy presented in Appendix 6 (Drg No. 8007/SK03) is providing all attenuation within a SUDS basin at the end of the network with a hydro brake flow control device to restrict the flows to the greenfield QBAR rate of 7.2 l/s. This is considered to be a 'end of pipe solution' and whilst it complies with standards for quantity it provides limited compliance to other criteria such as water quality, amenity and biodiversity. Therefore, incorporation of additional source control SUDS components such as water butts, permeable paving and bio retention (tree pits and rain gardens) will need to be considered further at detailed design stage.

#### 5.6 Foul Drainage

We are proposing to discharge all foul flows into the 450mm Dia gravity combined public sewer in Prestatyn Road to the north of the development. This will need to be discussed with Welsh Water who may identify an alternate point of connection.

Topography and proposed site levels design will allow for a gravity network to serve the entire development without any need for a pumping station.

#### 6.0 Conclusions and Recommendations

The site is located in Flood Zone 1 and has been shown to be at low risk of flooding from rivers, surface water, groundwater, sewers and climate change. However, there is potential of overland flows entering the site from Gronant Road at the southern end of the development. Highway drainage should be installed to intercept flows as mitigation.

All potential sources of flooding have been considered as part of this report. There are no known records of historical flooding at the site.

Further investigation on Gronant Road existing drainage should be undertaken and if necessary new drainage for Gronanat Road will need to be incorporated into the design to reduce the potential of overland flows from off-site sources post development.

Infiltration tests will need to be undertaken as part of the site investigation to determine if the underlying soils have favourable infiltration characteristics across the site for surface water flows to discharge to ground.

An 'option 2' for disposal of surface water has been developed with a discharge into the surface water public sewer network.

The development will increase the impermeable area of the site. This results in an increase in surface water runoff rates and volumes. In order to ensure the increase in runoff will not have an impact elsewhere flow restrictions and on-site attenuation will be incorporated into the design.

All surface water run-off from highways, roof and private drives will be collected into gravity piped networks, temporarily stored in a SUDS basin and will discharge at a restricted rate into the public sewer in Prestatyn Road.

Additional on-site source control components such as permeable paving and bioretention components (tree pits and raingardens) should be considered further at detailed design stage for compliance with the 5mm interception criteria.

The provision of trapped highway gullies, the SUDS detention basin and additional source control components will provide adequate treatment to surface water flows prior to discharge to the watercourse.

All foul sewers should be designed in accordance with Sewers for Adoption  $7^{th}$  Edition / Welsh Ministers Standards and will be subject to S104 Agreement.

A SUDS Maintenance and Management Plan should be produced to outline the activity and frequency of inspections and maintenance works required on any SUDS components subject to SAB Approval / Adoption.

This Flood Consequences Assessment and Drainage Strategy should be submitted to the Local Planning Authority in support of the planning application.

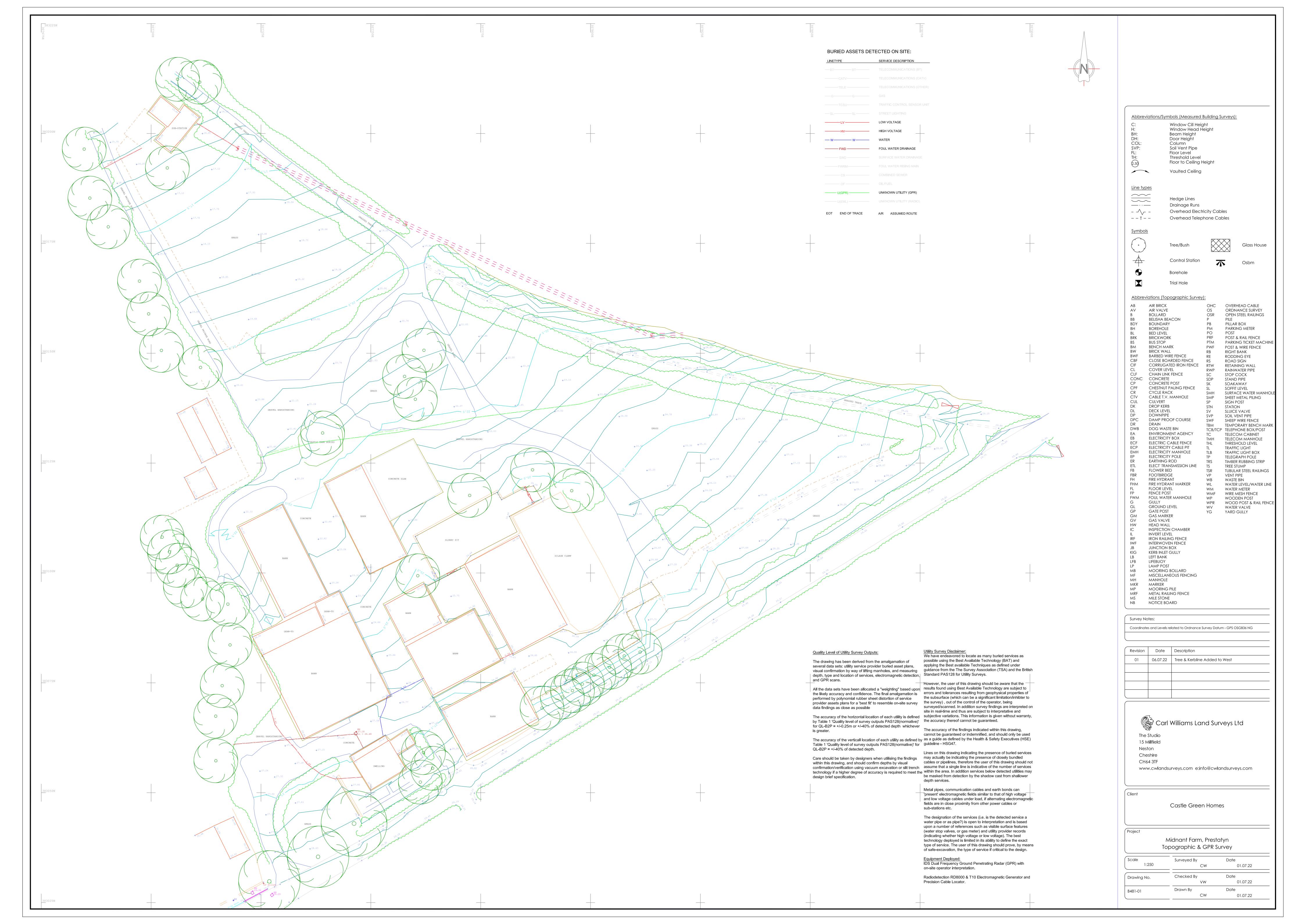
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## Appendix 1

## **Topographical Survey**

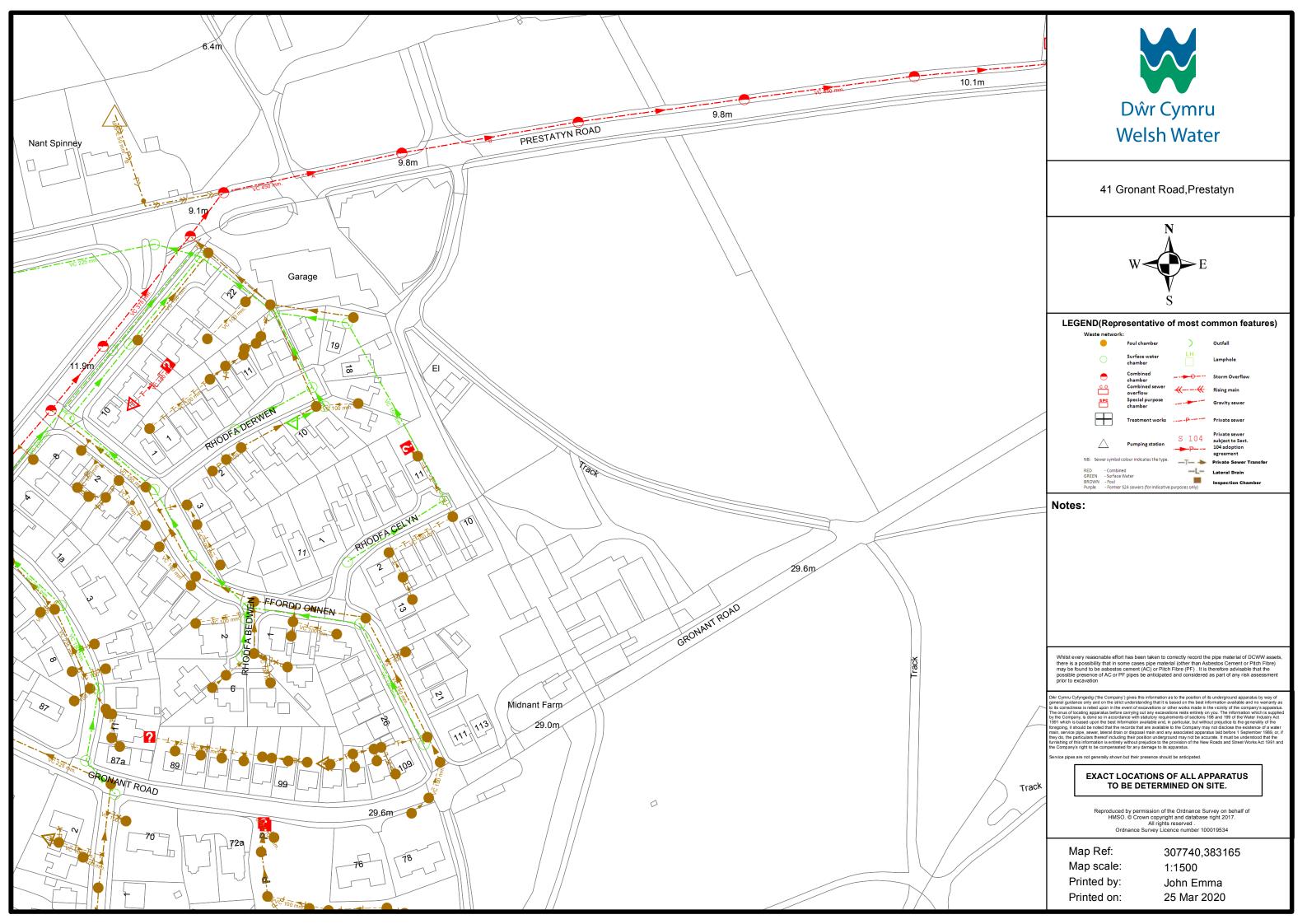
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## Appendix 2

## **Welsh Water Sewer Maps**

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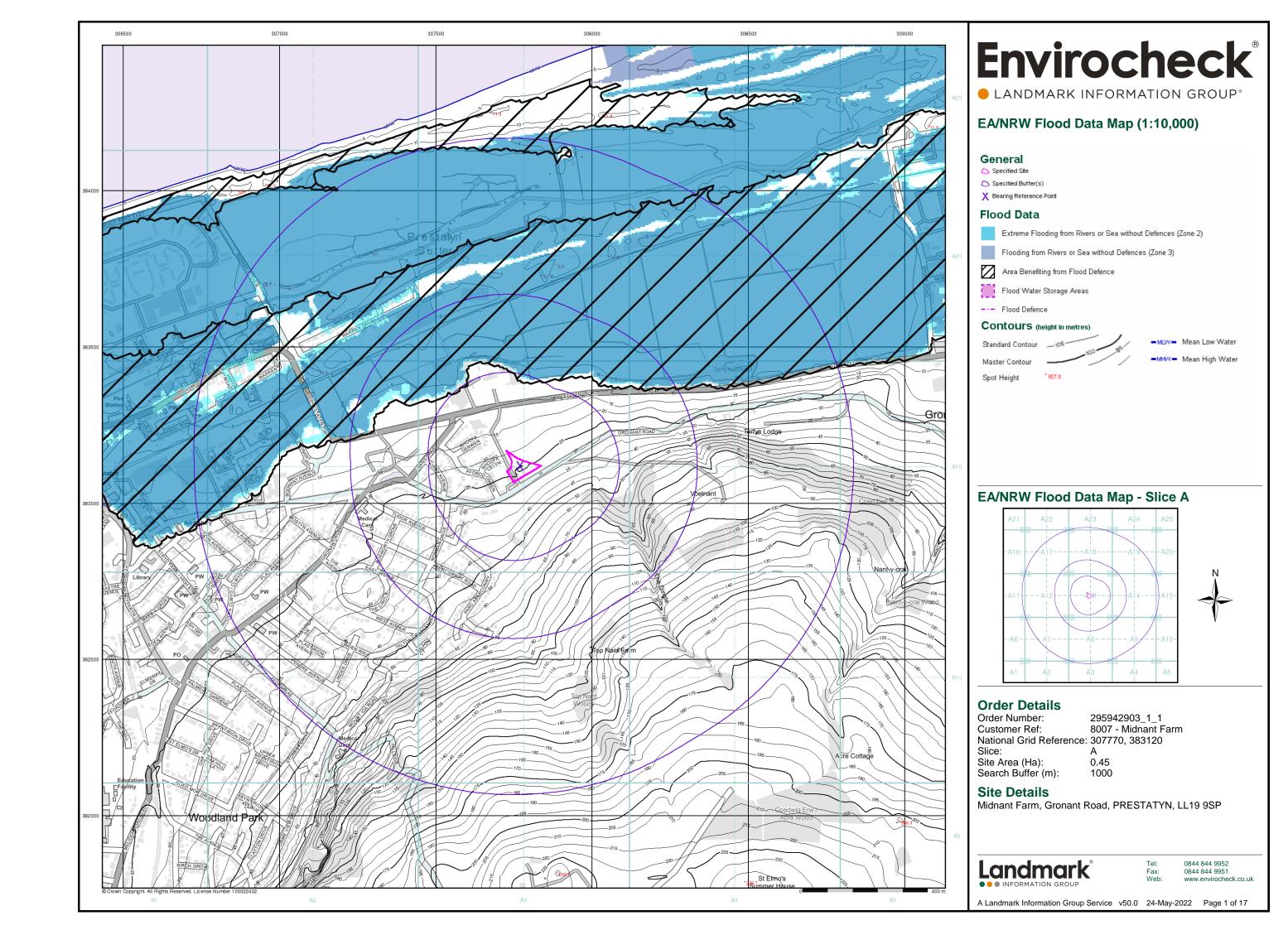


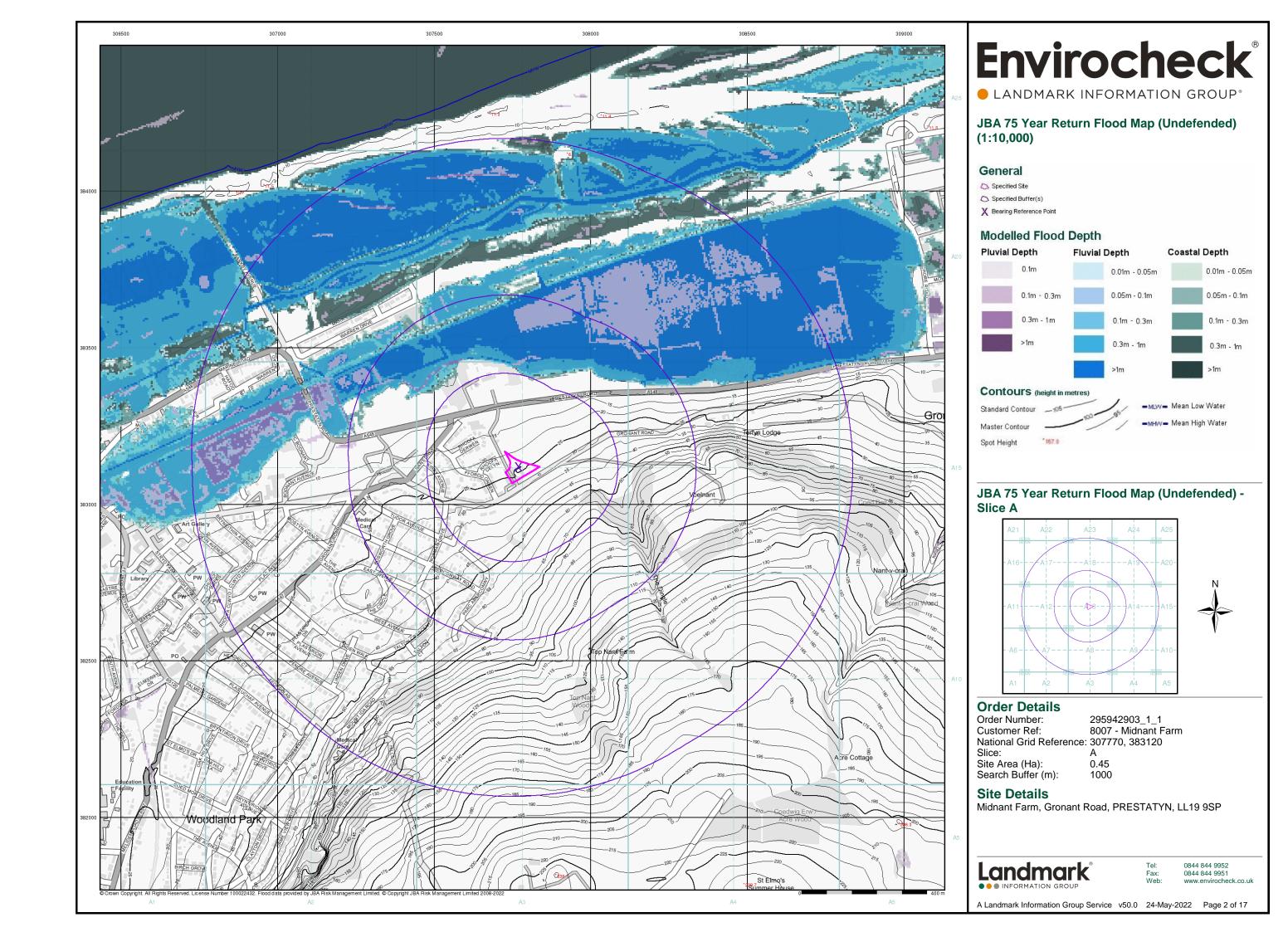
## Appendix 3

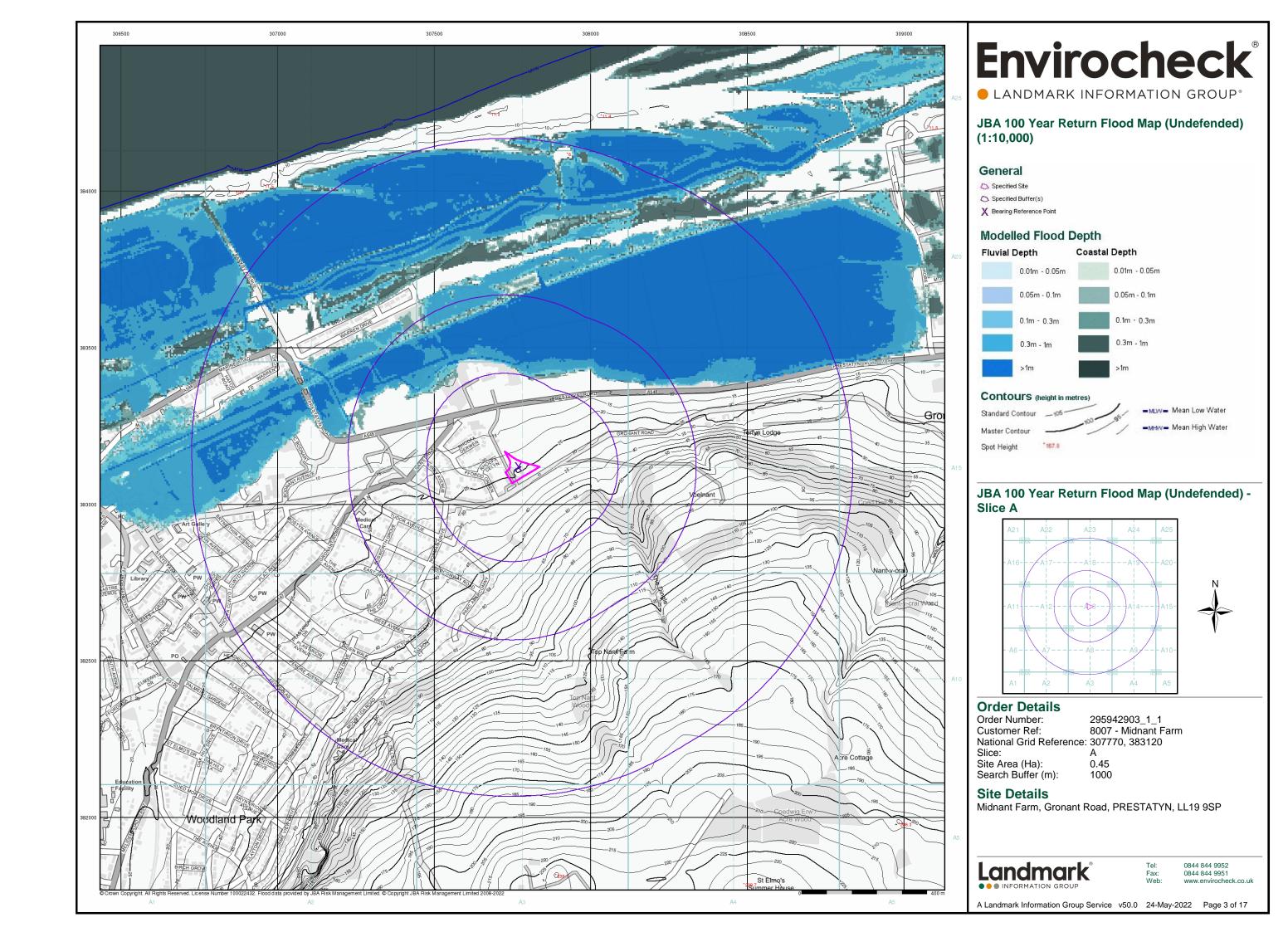
## **Enviocheck Flood Data**

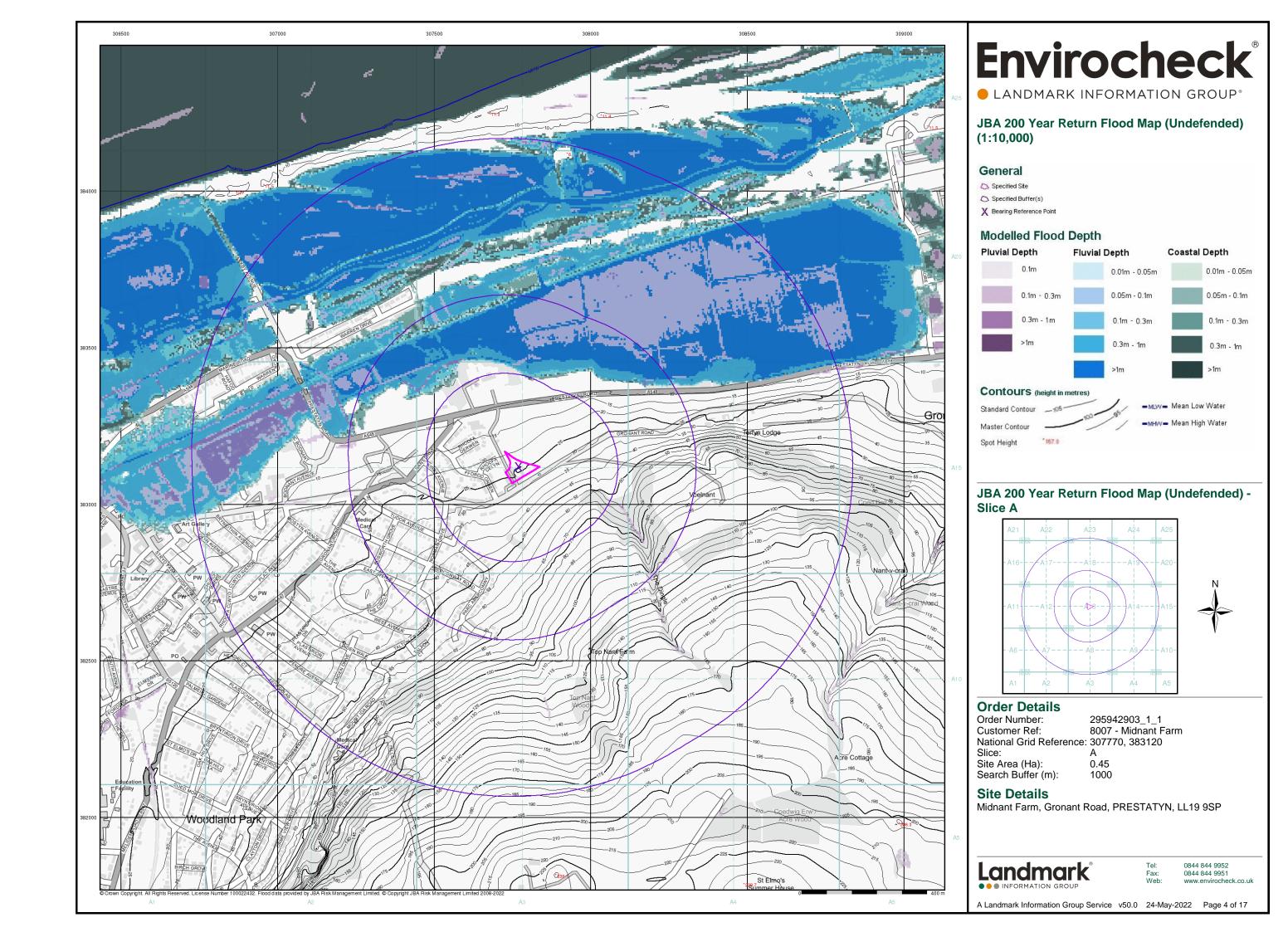
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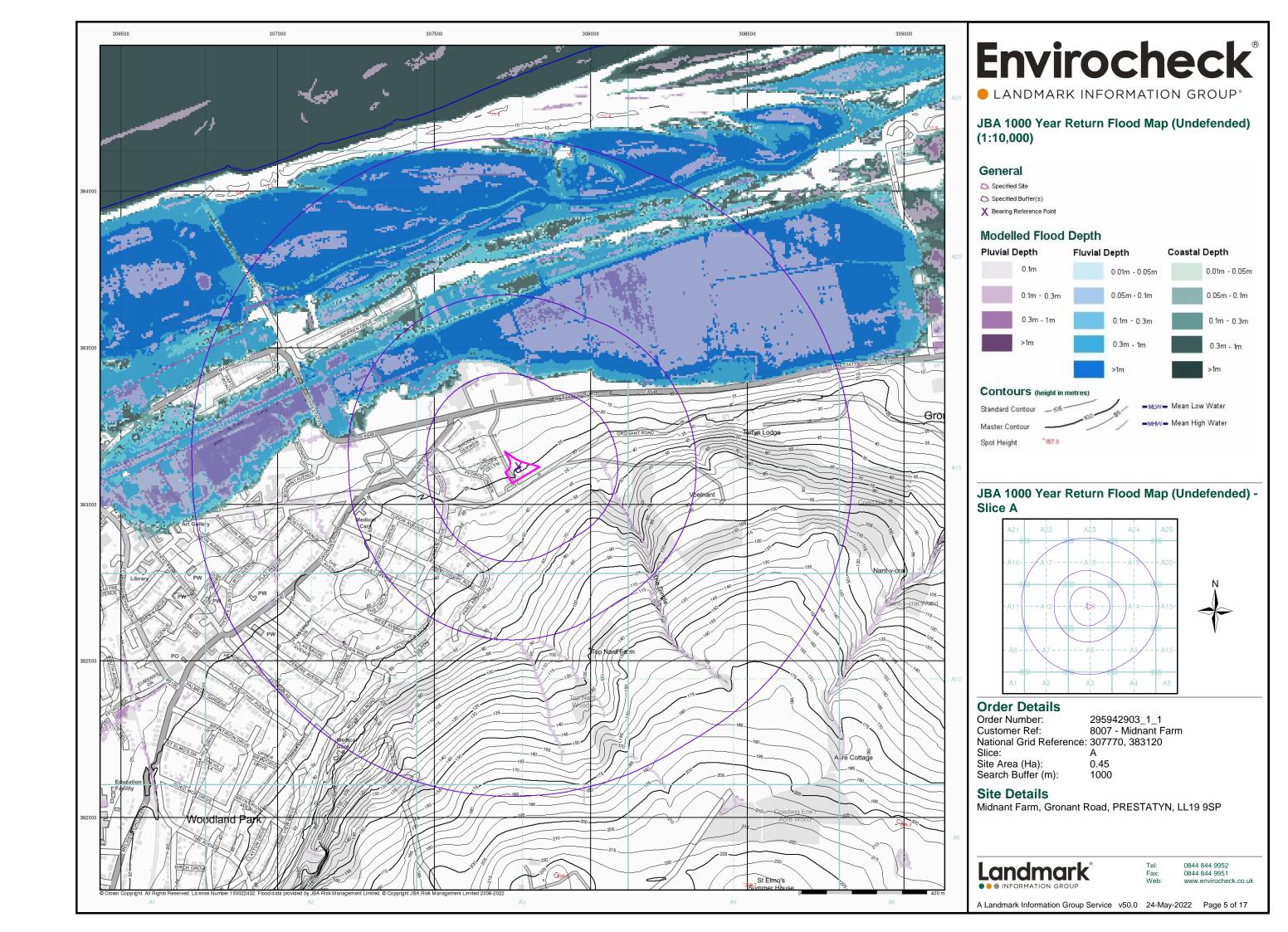
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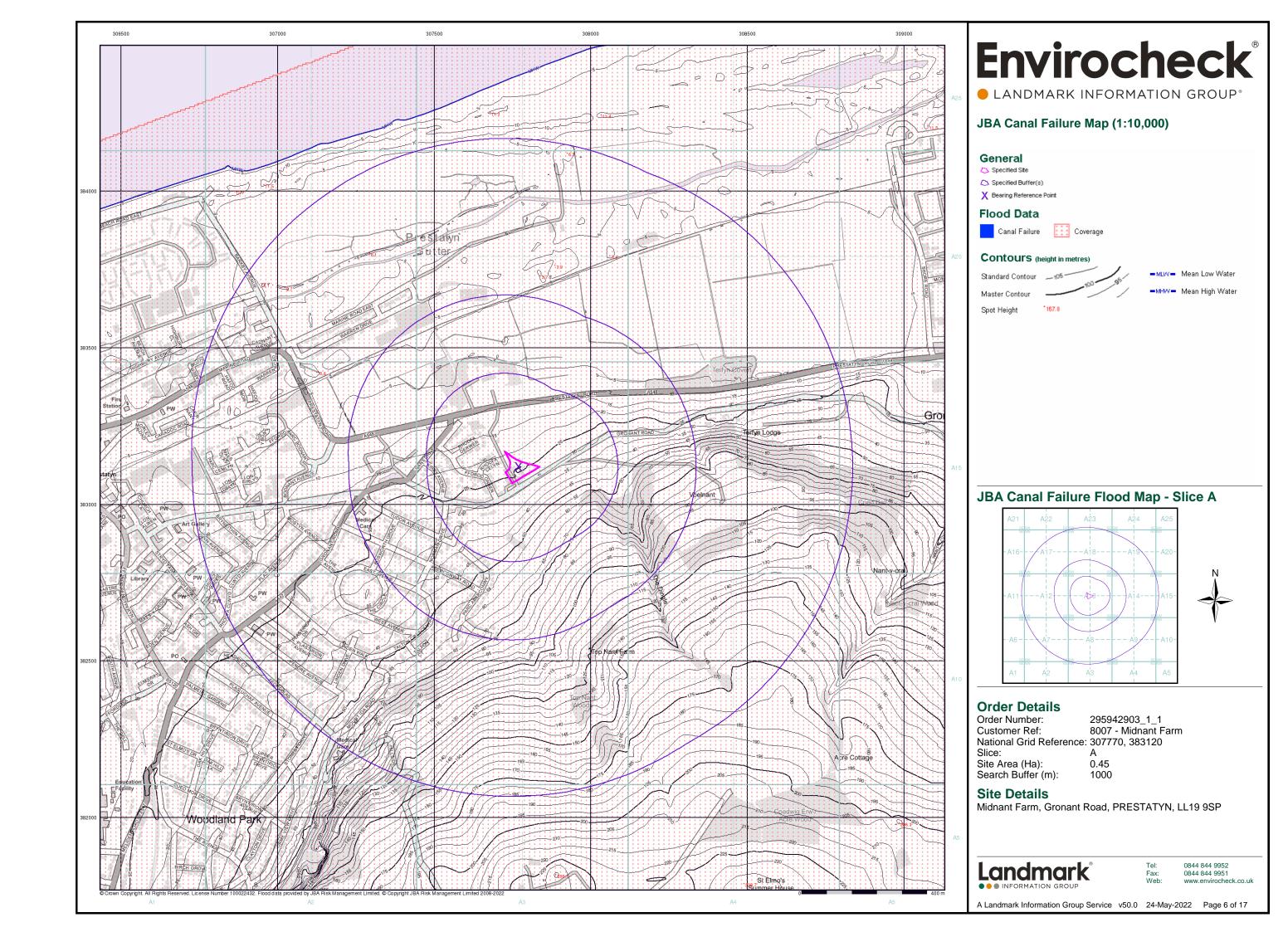


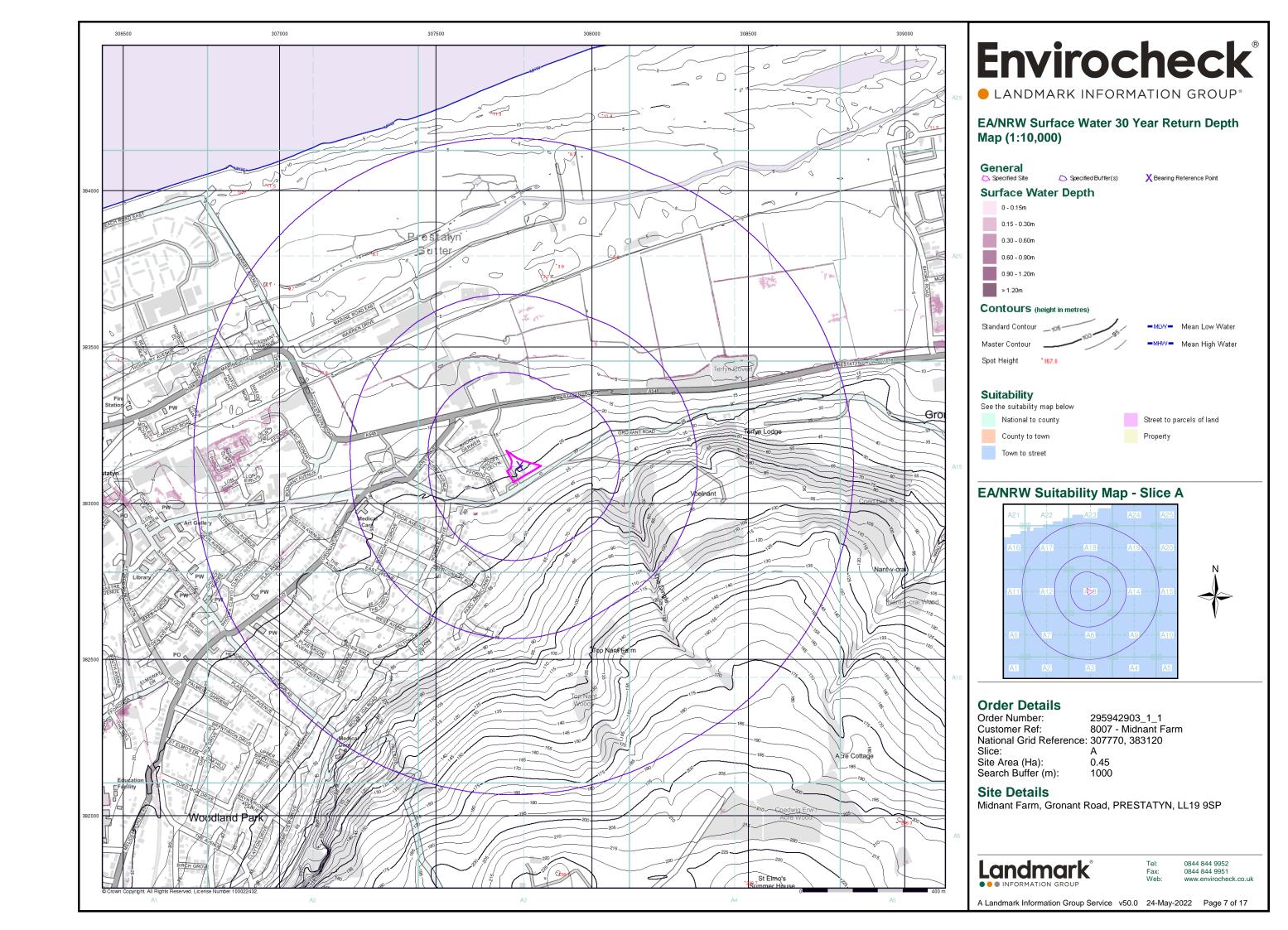


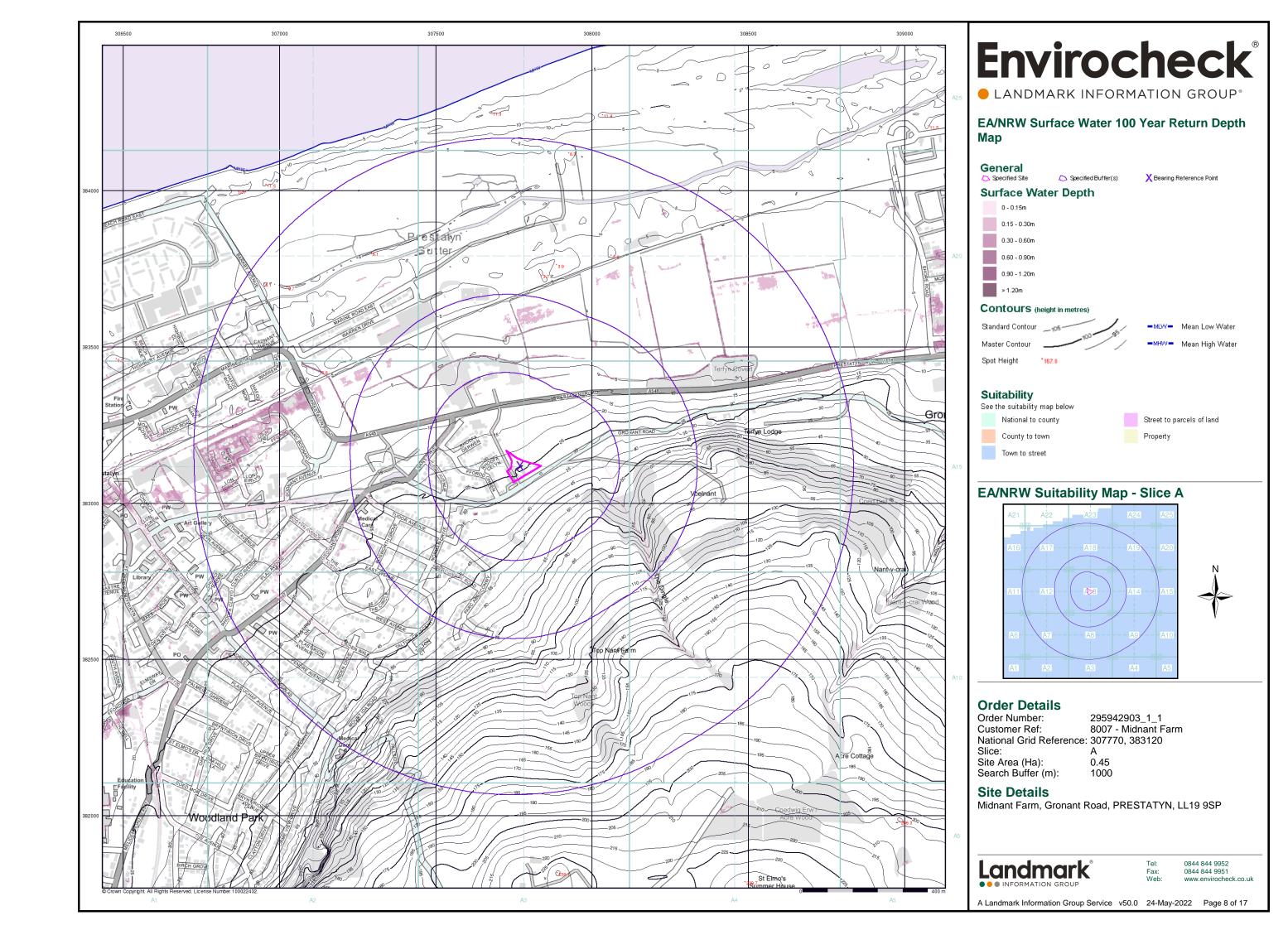


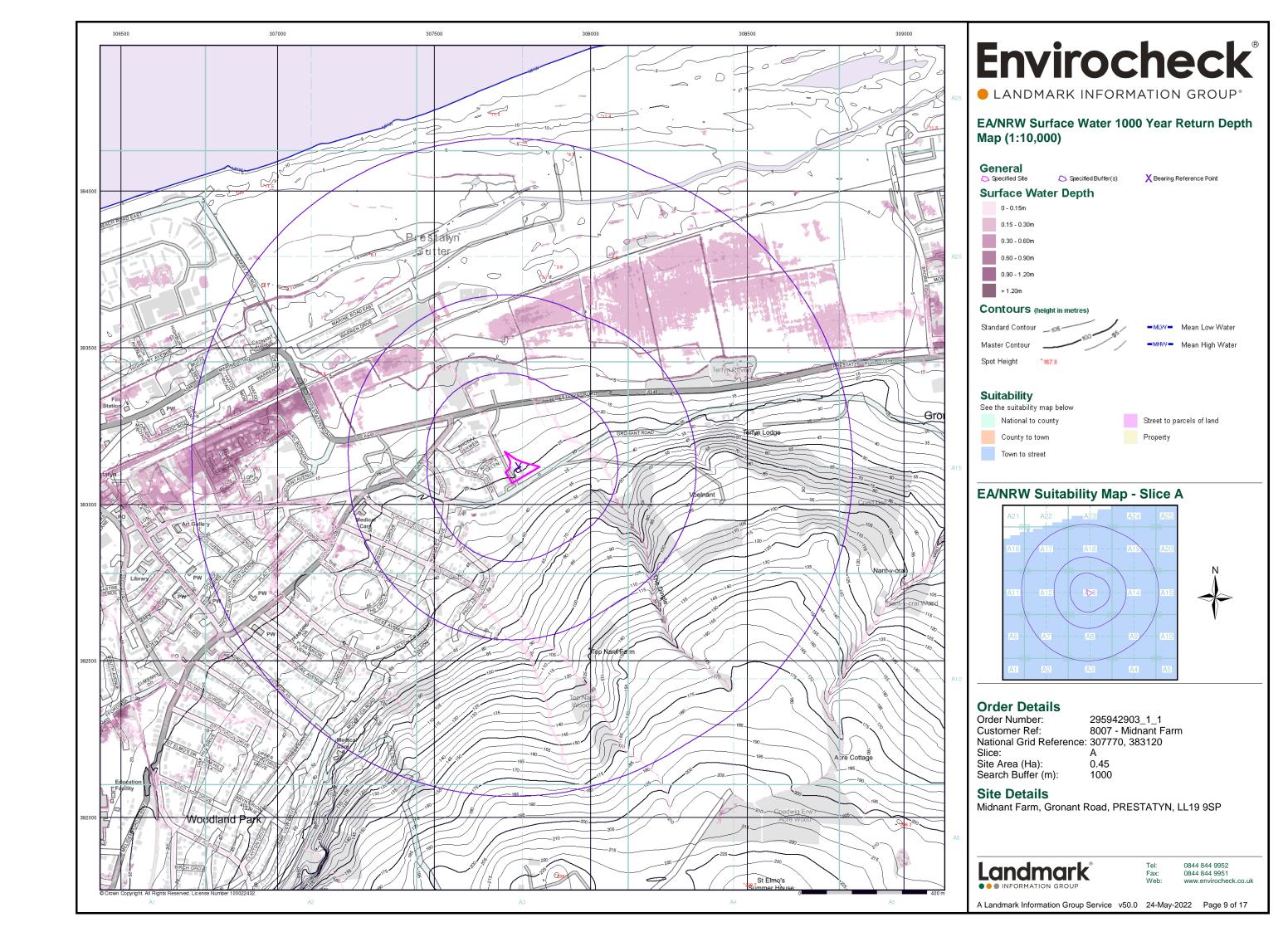


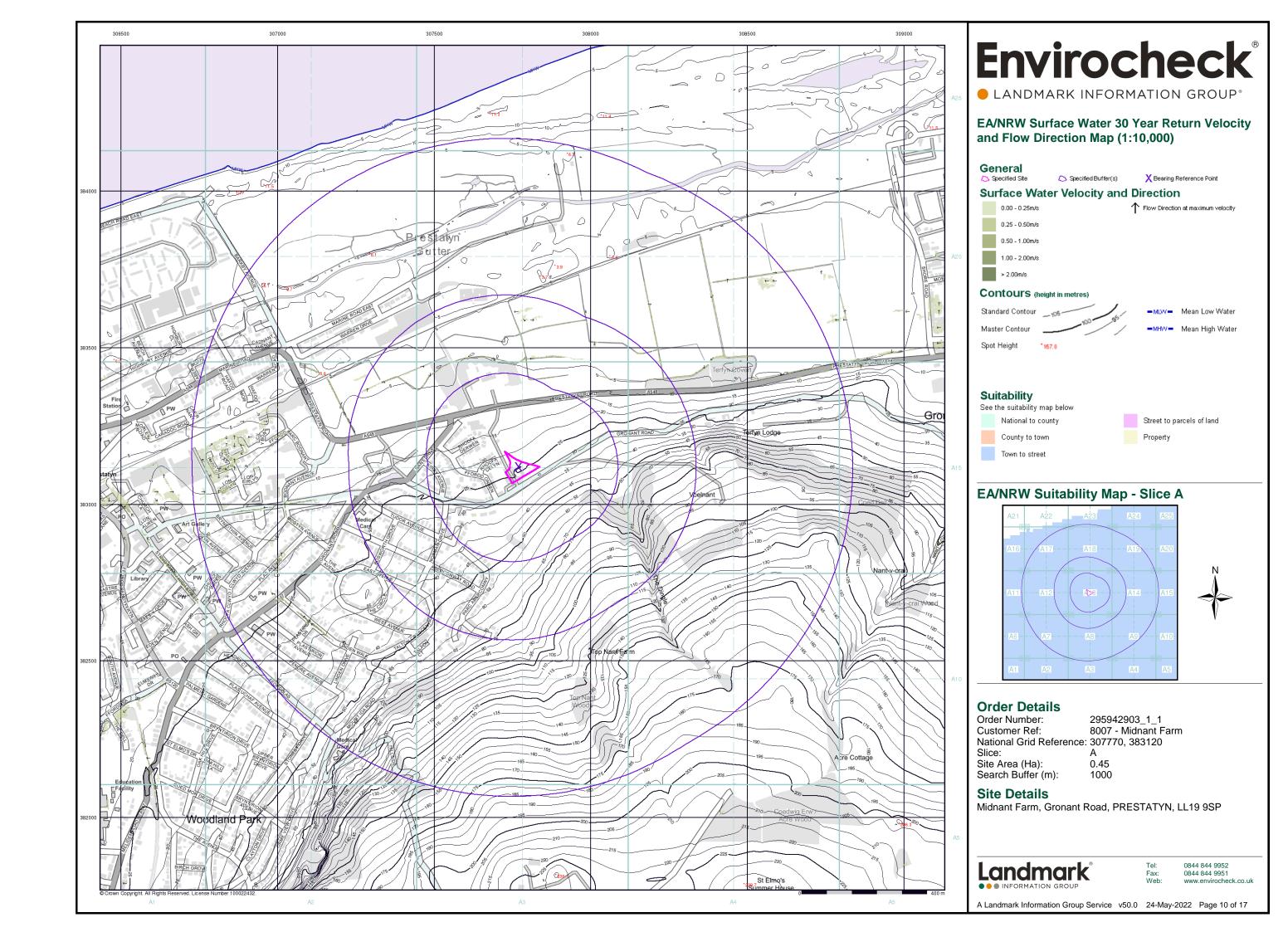


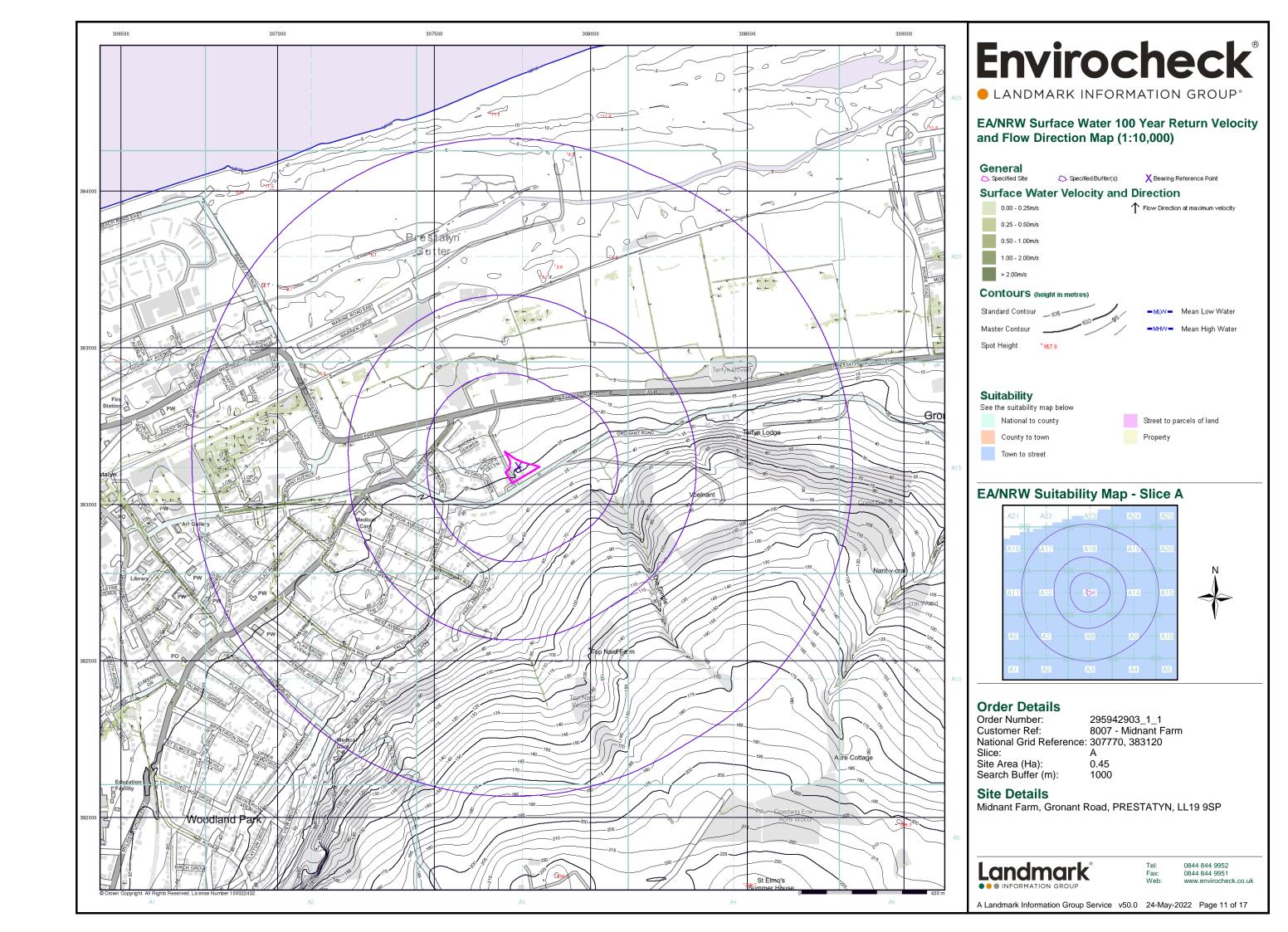


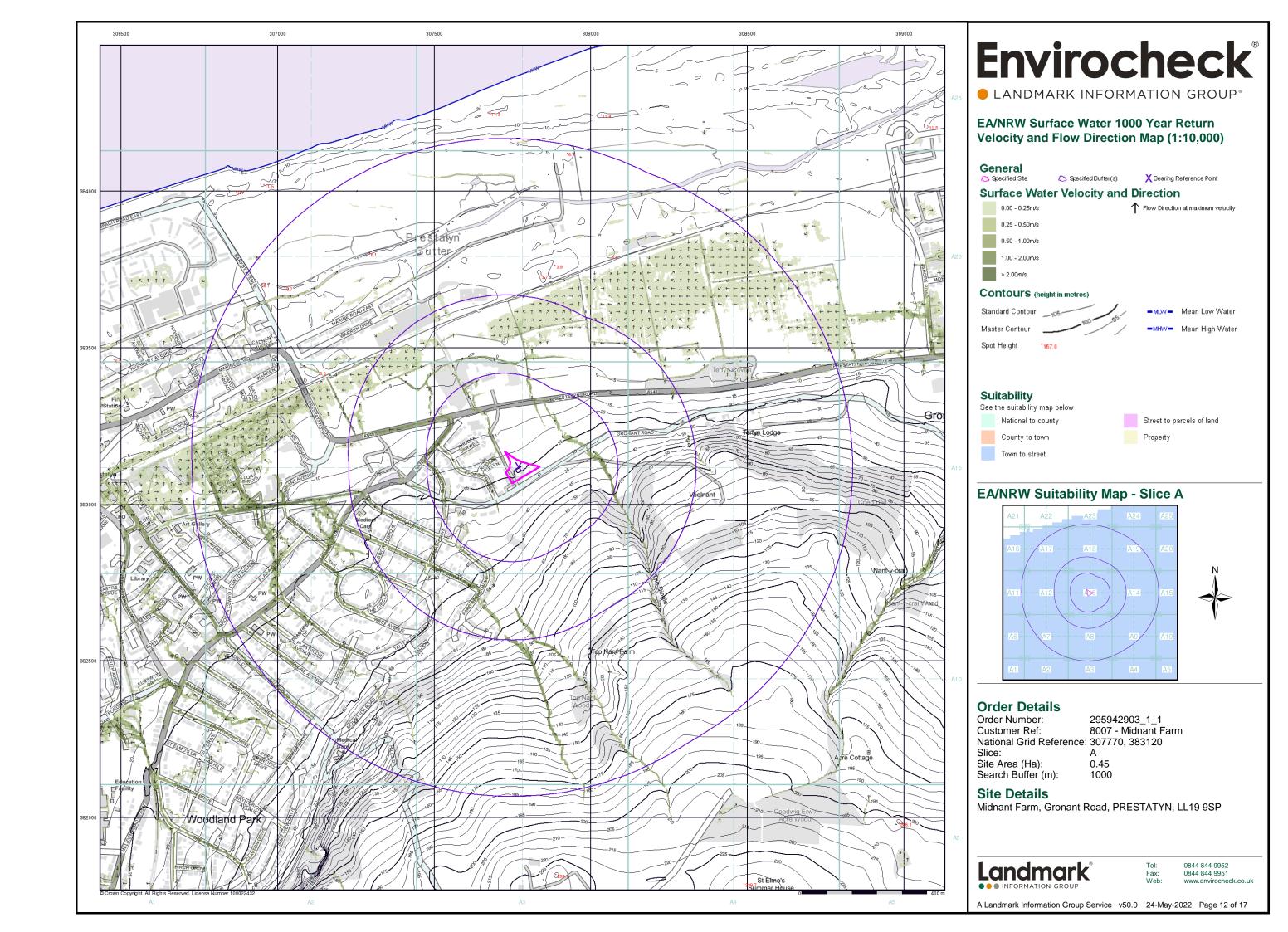


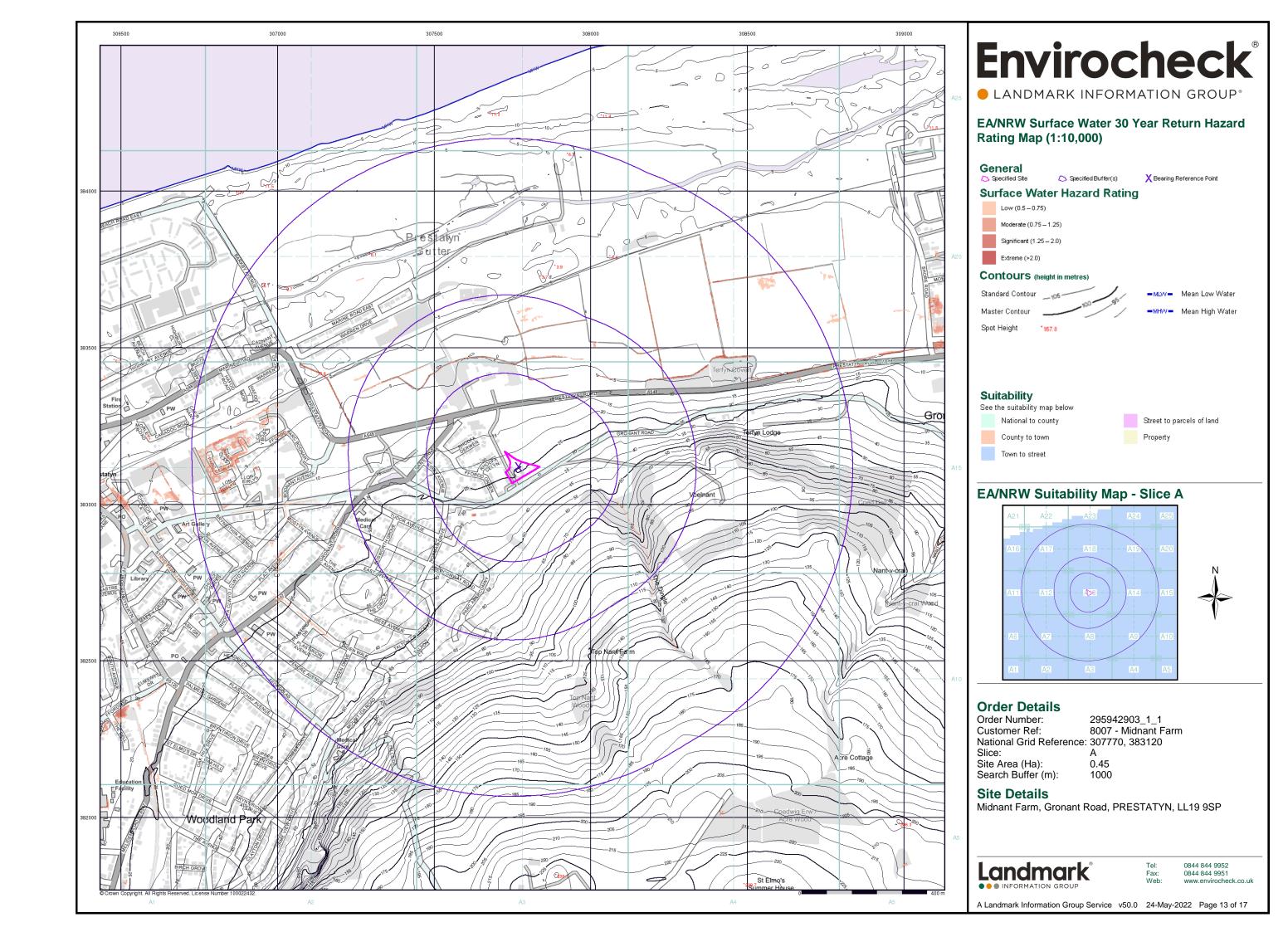


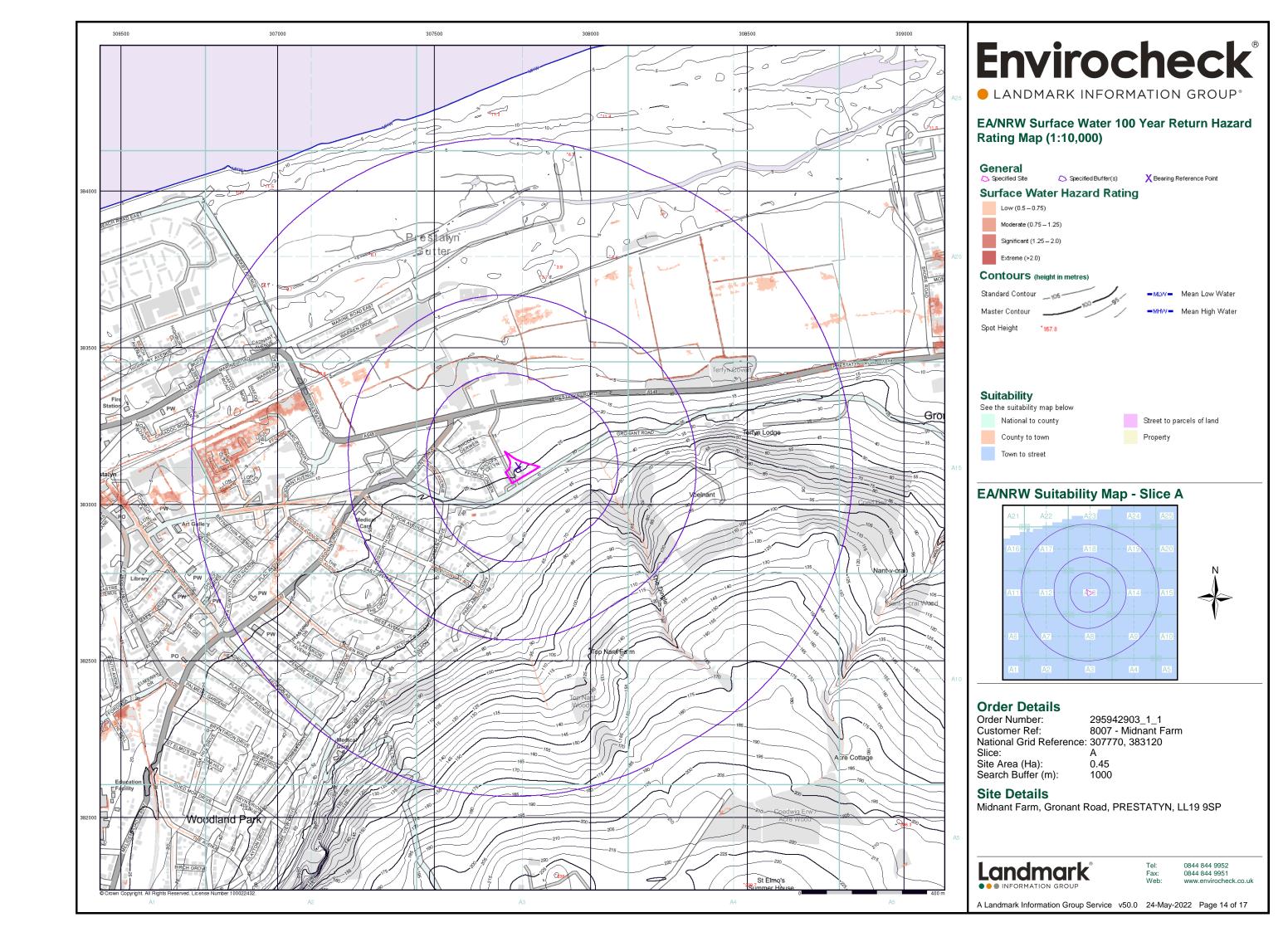


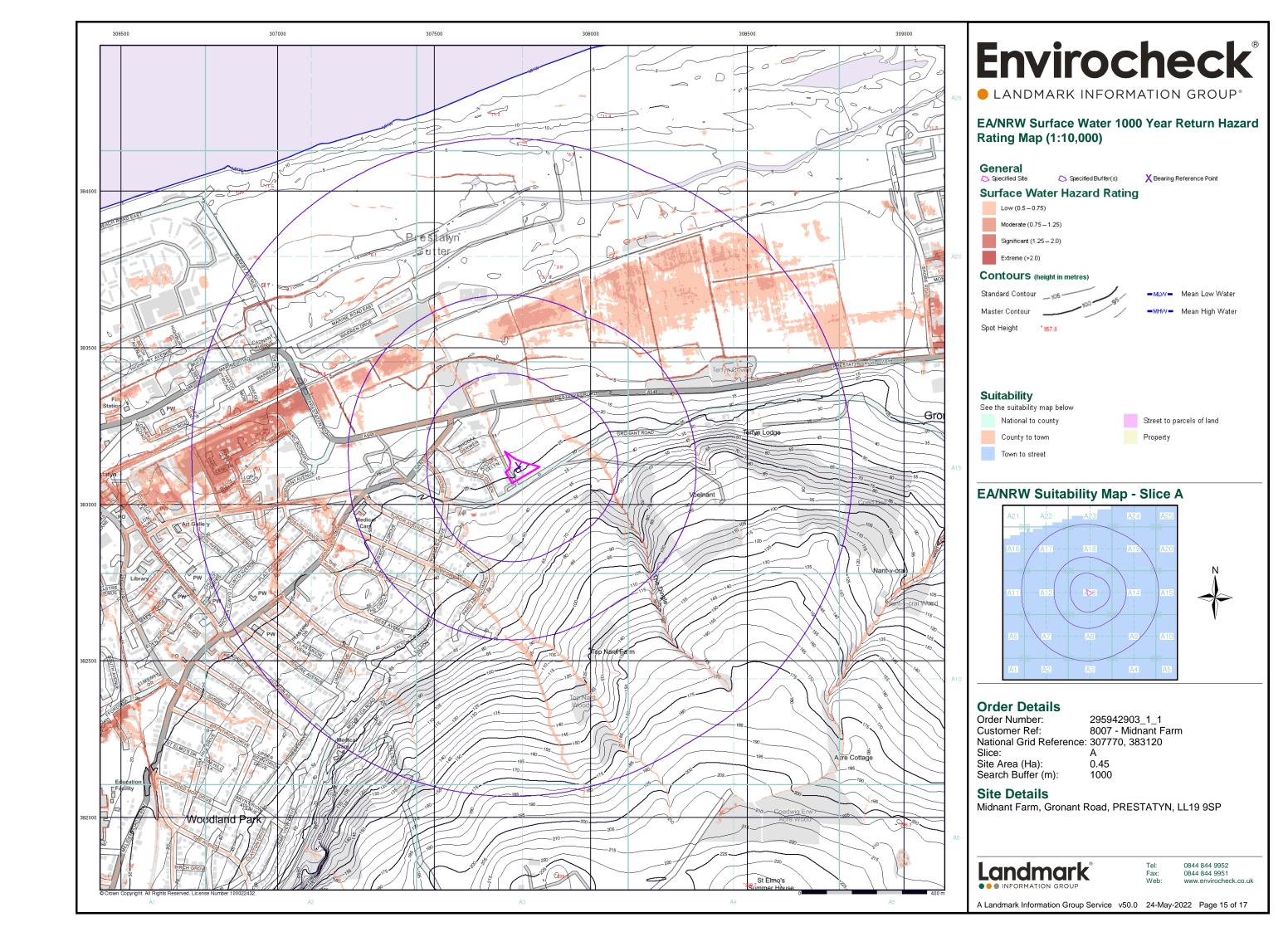


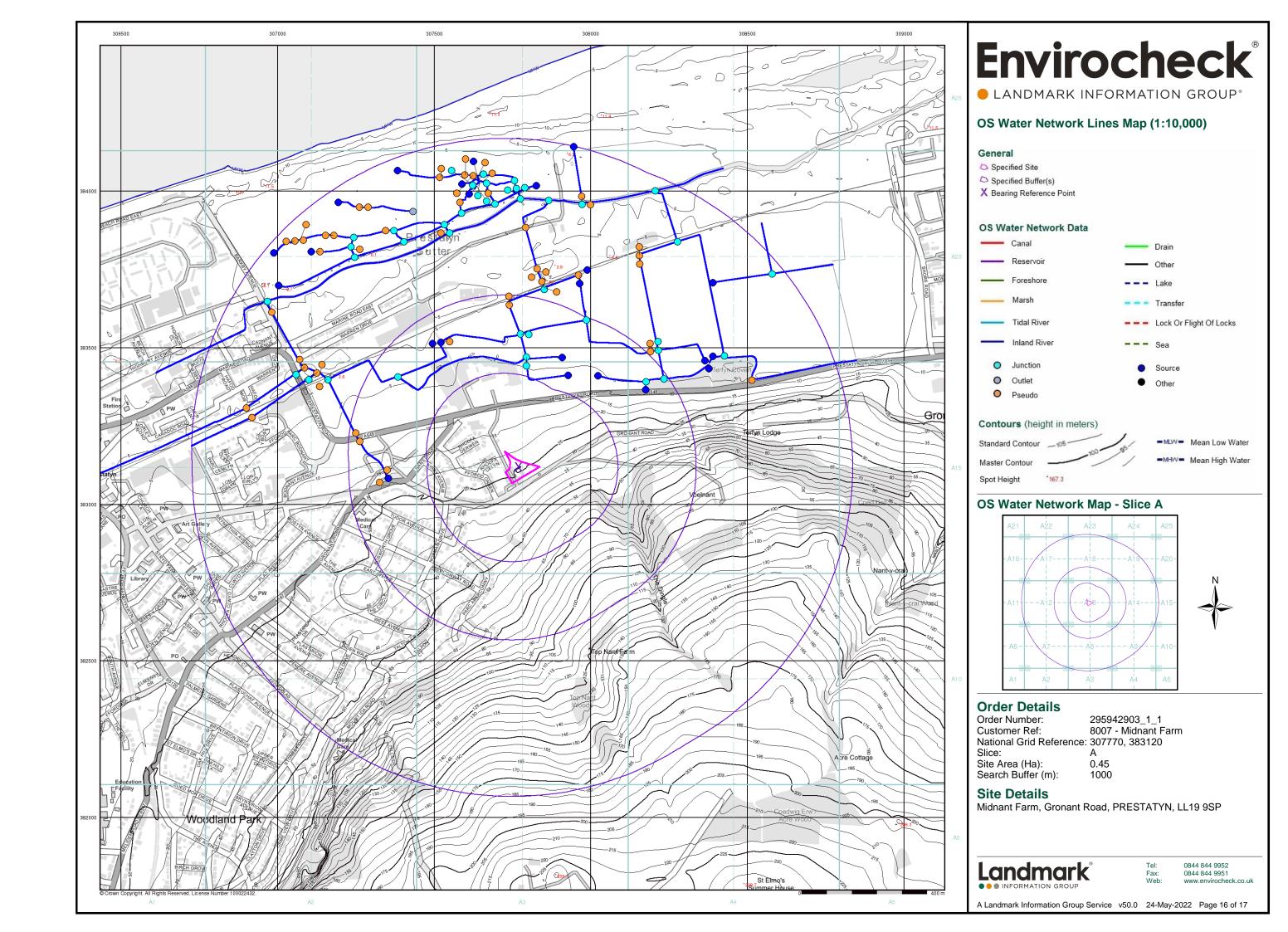


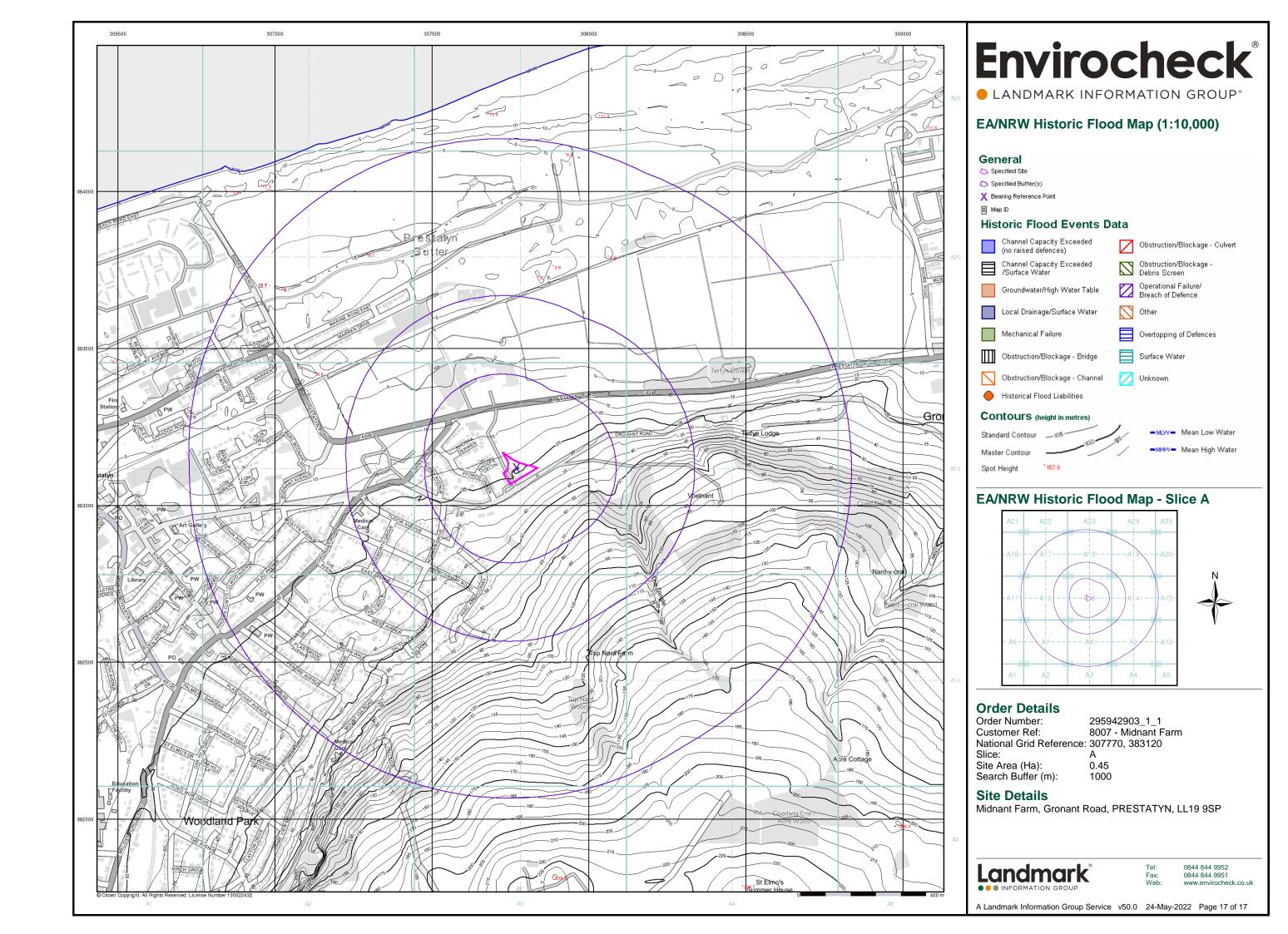


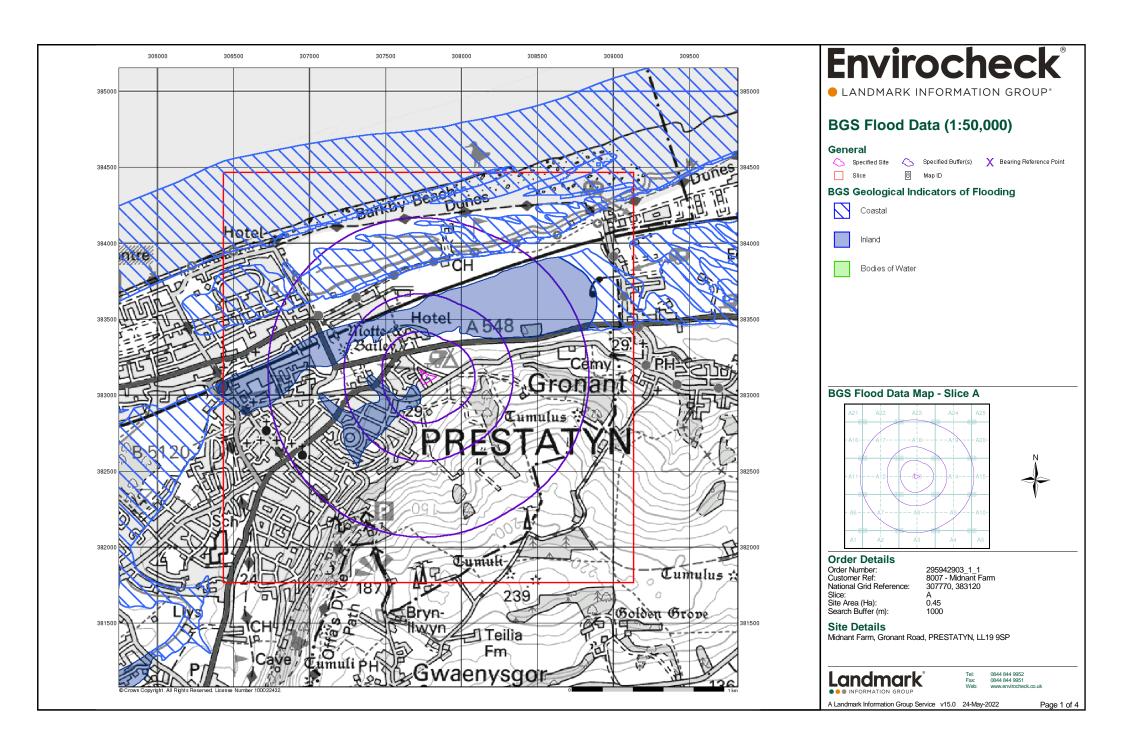


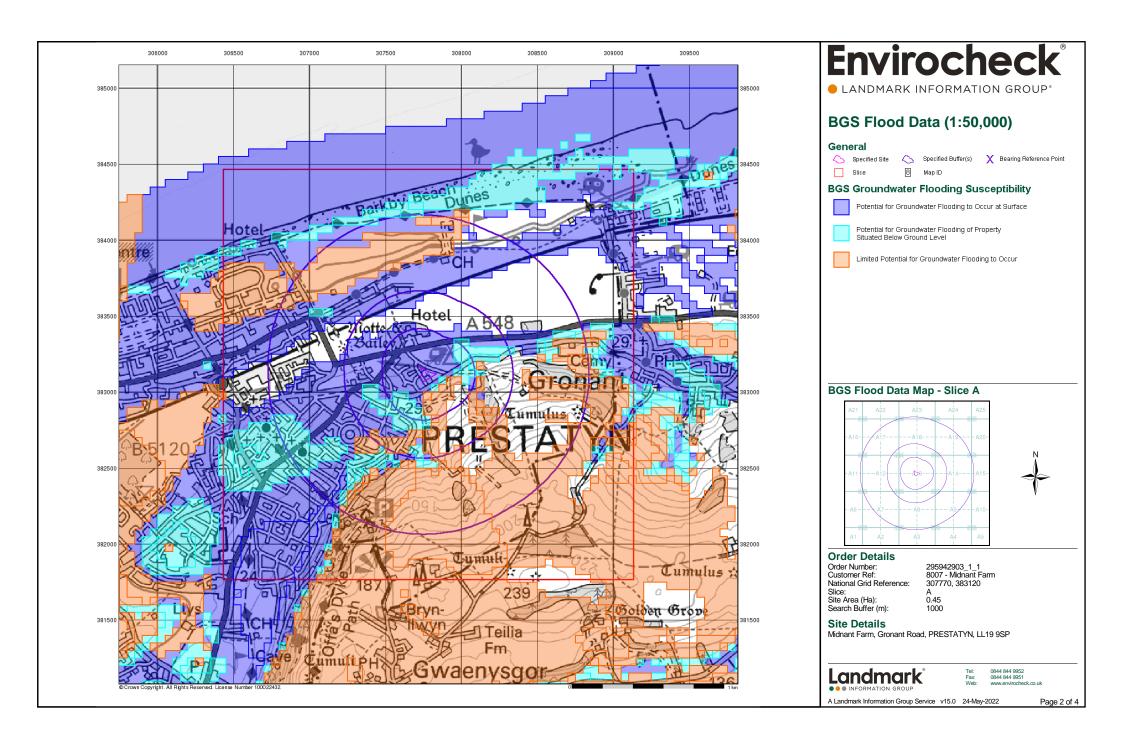


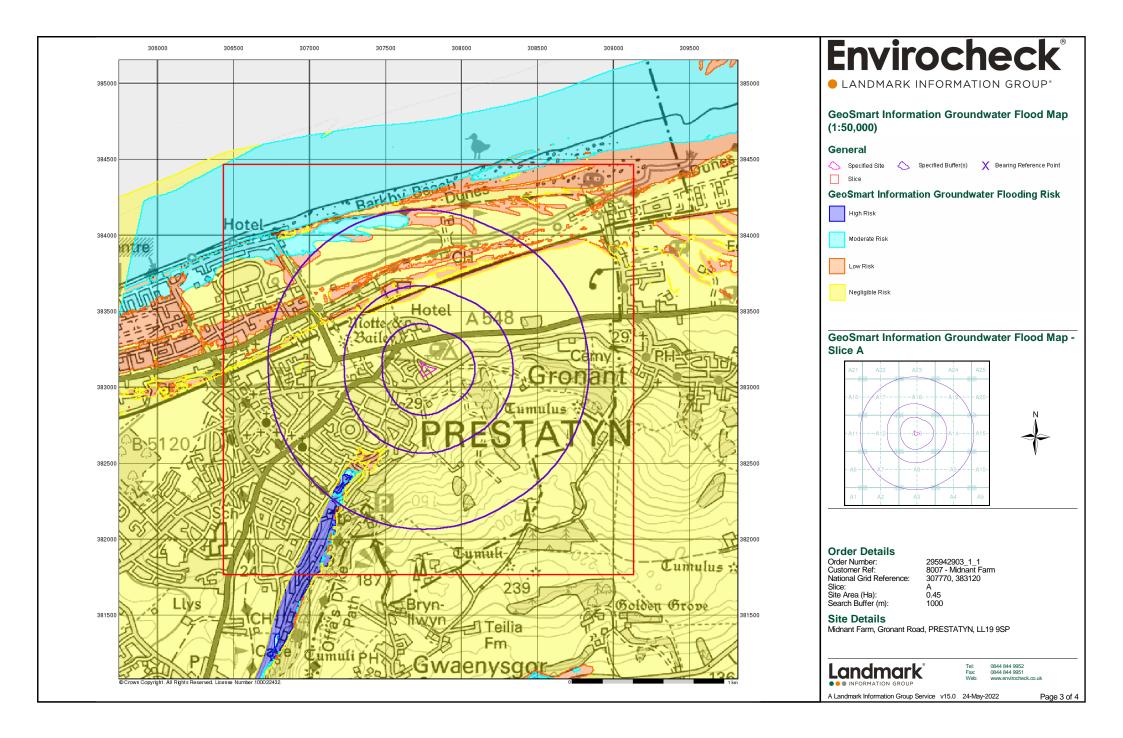


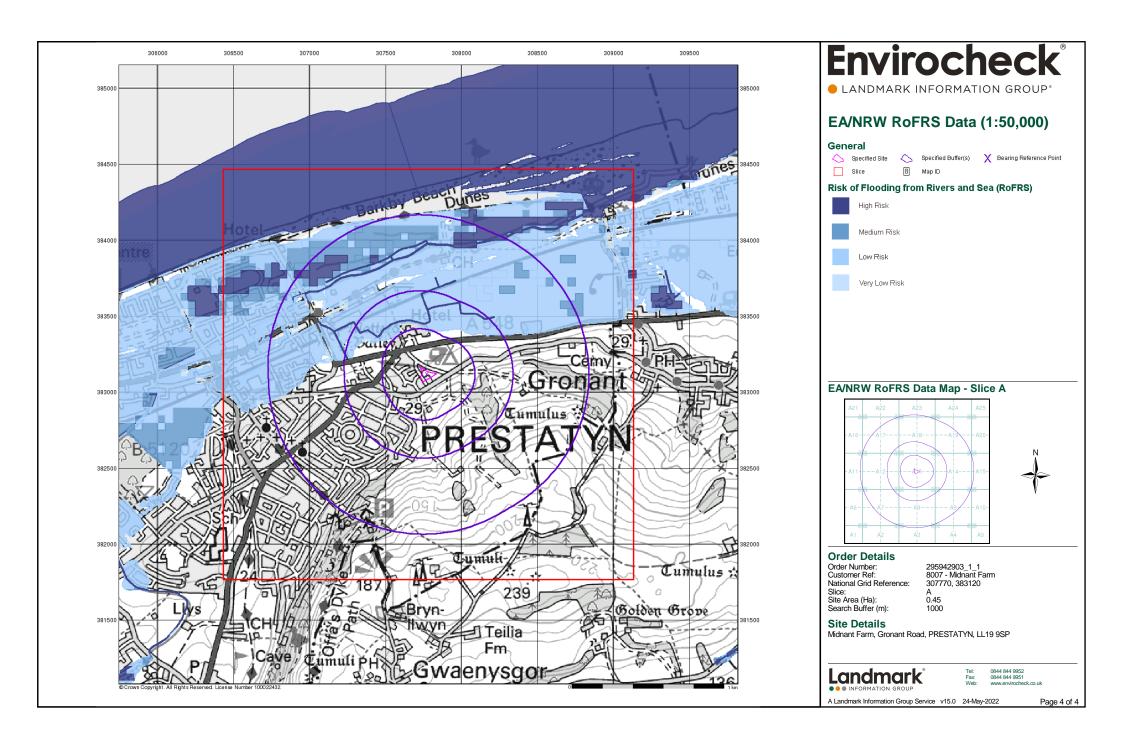












# Flood Consequences Assessment and Drainage Strategy Midnant Farm, Gronant Road, Prestatyn, Denbighshire

# Appendix 4

# **Infiltration Consideration**

**BGS** Information

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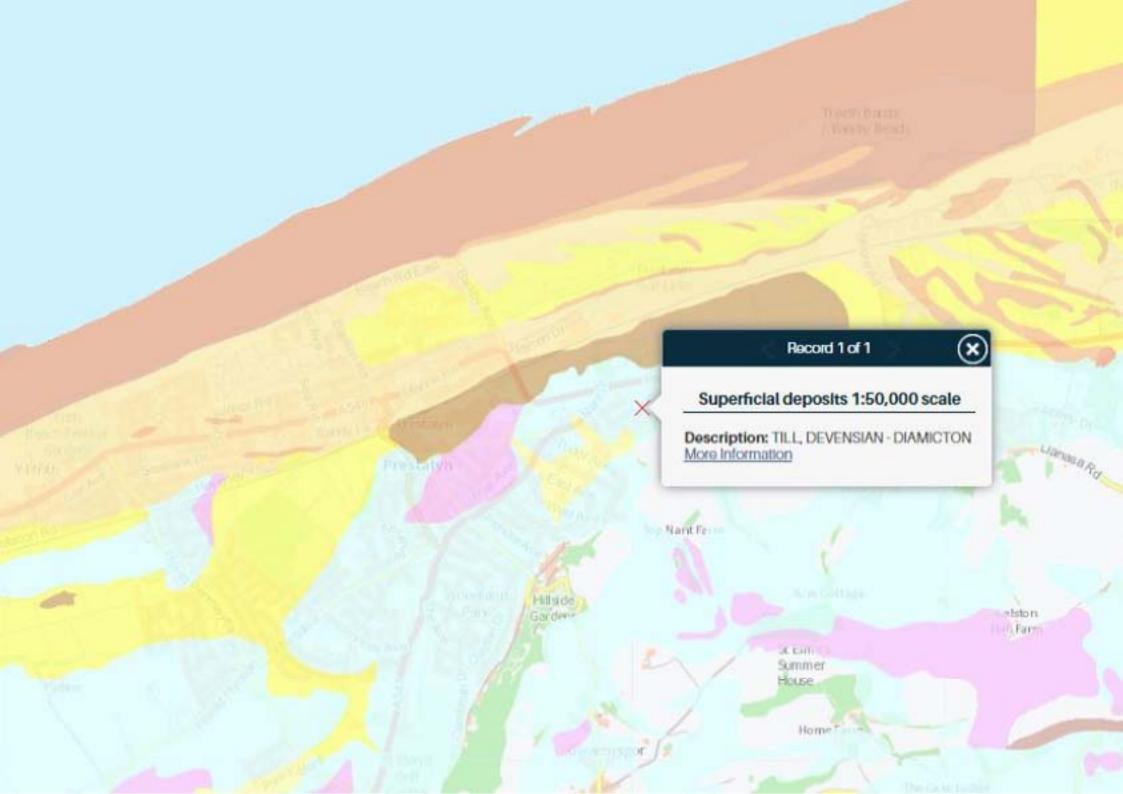


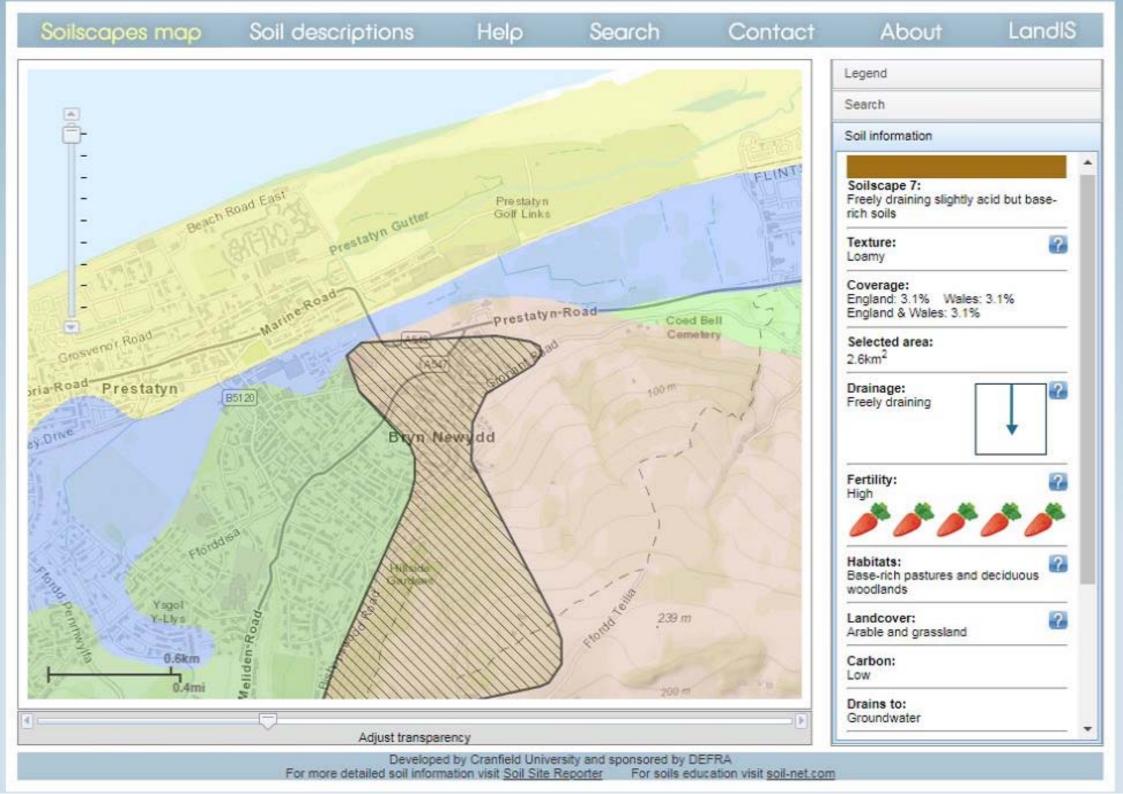
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		Grey, clayey, stoney SAND		0.0	0,95	7,55		0.40 - 0.85		
		Medium dense, red-brown, clayey, fine GRAVEL.	silty, SAND and	0000000			1.40	1.40 - 1.85	<b>1741</b>	
· .	British Geol	ngical Survey	British Geological Surve	0000			2.40	1.40 - 2.40 tish Geological Survey 2.40 - 2.85	#22 <sup>11</sup>	
				0.0.0	3,40	5.10	3.40	• 2.90 - 3.40		
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				888	10.00	-1.50		• 9.70 - 10.00		7.11.
		Type of Sample  ☐ Undisturbed ● Bulk  Is S.P.T. X Vane  Ic. C.P.T. △ Water	Remarks (Observatio Water Balance used: Standing 4.50m Standing 5.30m Standing 7.00m	7.]]	.74 •74 •74 •74 •74	8.00 a. 9.00 a. 9.15 a. 2.00 a.	m. m.	Casing 5.00 m Casing 6.50 m Casing 10.00 m		
			Water levels are subject t	o seasonal	or tidal v	ariations	and should	not be taken as con-	stant	
	- 5 2	· · · · · · · · · · · · · · · · · · ·			<del></del>					

		Но	lst Soi	I En	gine	eeri	_			1	<b>17</b>
	Contract No.	S1 1727/F2974	E	BORE	HOLE	LC	G	STOS Shee	SE , 43 at 1 of]	<u> </u>	
	Client . Clayd C Method of Bor Diameter of B	ounty Council		eological Surv on)	ey			Chai Grou Date	1.0.D.	1.0.D. &1	
	D	escription of	Strata		Legend	Depth Below G.L.(m)	O.D. Level (m)	Casing Depth at Sampling	Sampling and Coring	"N"/ R.Q.D.%	Daily Progres
	TOP SOIL					0,90	6.40		• 0.90		
L	oose, brown, cl	ayey, stoney S	AND.		0000	2,00	5.30	1.00	1.00 - 1.45		
Geologi <u>c</u>	oose, well grad	ed, silty, cla	iyey SAND and G	RAVEL 3 Sun				2,50	2,50 cal Survey		
	Medium dense,	wel] graded, s	andy GRAVEL			3,40	3.90	4.00	4.00 - 4.45	#70# \	
			C C C C C C C C C C C C C C C C C C C	5.40	1.90	_					
Genlog <b>P</b>	ense, brown, cl	ayey, stoney S	S <b>AND.</b> British G	eological Sun	0000			5,50	5,50 - 5,95 tigh Geological Survey		
					0 8	7.00	0.30	7.00	7.00 - 7.45	-	1.11.74
Geologic	al Survey		British G	eological Sun	SILCO SI SILCO SI SILCO SI SILCO SI SILCO SI SI SILCO SI SI SI SI SI SI SI SI SI SI SI SI SI			(IIIII)	Iritish Geological Survey		
	Type of S	Sample	Remarks (Ob	servatio	ns of Gr	ound W	ater etc.	.)		I	
	Undisturbed	Struck	3,40	1.11.	74	2.3	0 p.m.	Casing 3.00	m		
Is	S.P.T. C.P.T.	X Vane Δ Water	Sealed	5.00	1.11.	74	3.3	0 p.m.	Casing 5.00	8	
Ic.	Citali	77 Anglet	]								

Holst Soil En	gine	eri	ng l	Limi	ted		ehole N
Contract No. S1 1727/F2974 BORE Location Prestatyn A.548	HOLE	LC	)G	Shee	SE, 44. et 1 of?. mage 468.00 m		
Client Clwyd County Council. House Geological Sur Method of Boring Shell & Augar (Percussion) Diameter of Borehole 0,15 m	VC)			Grou Date	1	0.D.	
Description of Strata	Legend	Depth Below G.L.(m)	O.D. Level (m)	Casing Depth at Sampling	Sampling and Coring	"N"/ R.Q.D.%	Daily Progre
TOP SOIL		1,00	5.00				
Brown, clayey,stoney SAND	2/9/16	1,20	4.80	0.00	П		
Loose, fine GRAVEL	000000			2.00	1.00 - 1.45	#70#	
ilogical Survey British Geological Sur	(2000)	3.00	3,00		762,00 - 2,45 mish Geological Survey		
Medium dense, well graded, sandy GRAVEL				3,00	3.00 - 3.45	ייירויי	
				4.50	4.50 4.50 - 4.95	#18#	
logical Survey British Geological Sur	0.0000000000000000000000000000000000000			6.00	6.00 - 6.45	#2 <b>]</b> #	
Medium dense, medium-coorse GRAVEL	00000	7.60 7.90	-1.60 -1.90	7.60	7.60 T <sup>c</sup> 7.60 - 8.05	#22#	
Medium dense, course GRAVEL and SAND	2000			-	1.60 - 8.05		
Continued:							
ogical Survey British Geological Sur	1 1 1 1 1 1 1 1 1 1 1 1 1 1 1 1 1 1 1				Eritish Geological Survey		
Type of Sample  ☐ Undisturbed ● Bulk Is S.P.T. X Vane  Remarks (Observation Struck 1.90 m Standing 1.50 m Standing 2.30 m	ons of Gr 30.10 31.10	0 <b>.74</b> 0.74	12.2 8.0	.) 20 p.m. 00 p.m.		.50 m	
Ic. C.P.T. △ Water  O Jar □ Piezometer Water levels are subject	to seasona	l or tidal	variations	and shoul	d not be taken as co	nstant	





# Appendix 5

# Correspondence

Dwr Cymru Welsh Water Historical Flooding

Natural Resources Wales Historical Flooding

Denbighshire County Council Historical Flooding

Our Ref: 8007\_FCA 25 November 2022

### **Andy Jones**

From: Environmental Information Requests <EnvironmentalInformationRequests@dwrcymru.com>

**Sent:** 29 June 2022 09:48

To: Andy Jones

**Subject:** RE: FCA Historical Flood Information

Our Reference: EIR/1169/2022

Dear Mr Jones

### **Request for Information**

RE: 8007 Midnant Farm, Gronant Road, Prestatyn

We write further to your request for information dated 7<sup>th</sup> June 2022, which we have been considering under the Environmental Information Regulations 2004.

We can confirm that we have reviewed our flooding database and we have no flooding history within the location or vicinity of the proposed site.

Turning to your query on specific drainage requirements, we have a dedicated Planning Team within our Developer Services function that can provide advice on the capacity in our infrastructure to accommodate new development.

Our pre-planning service will review the capacity available in our sewerage network, wastewater treatment works and clean water network to accommodate the flows/demand from a proposed development. It will also raise awareness of any of our assets crossing the site which will need to be factored into the layout of your development and provide general advice on surface water drainage.

An application for pre-planning advice can be submitted online via the following address <a href="https://developers.dwrcymru.com/en/applications/planning/pre-planning">https://developers.dwrcymru.com/en/applications/planning/pre-planning</a>.

If you wish to discuss in more detail our Planning Team are contactable on 08009172652 or alternatively by email <a href="mailto:developer.services@dwrcymru.com">developer.services@dwrcymru.com</a>.

We hope that this response is clear. Should you have any questions, please contact us by email at EnvironmentalInformationRequests@dwrcymru.com.

If you are dissatisfied with the handling of your request, you have the right to ask for an internal review. Internal review requests should be submitted within 40 working days of the date of receipt of this response and should be addressed to Company Secretary, Linea, Fortran Road, St Mellons, Cardiff CF3 0LT

If you are not content with the outcome of the internal review, you have the right to apply directly to the Information Commissioner for a decision.

Yours faithfully,

#### Dŵr Cymru Welsh Water

From: Andy Jones <aiones@coopers.co.uk>

Sent: 07 June 2022 16:17

To: Sewerage Services <Sewerage.Services@dwrcymru.com>

Subject: FCA Historical Flood Information

\*\*\*\*\*\* External Mail \*\*\*\*\*\*

# 8007 Midnant Farm, Gronant Road, Prestatyn SJ077831 X=307750 , Y=383100

#### **FCA Historical Flood Information**

To whom it may concern

We are undertaking a Flood Consequences Assessment and Drainage Strategy for the above site (see attached Site Location Plan) and request any information you may have in relation to historical flooding or any information you may consider relevant to assist with the production of the FCA report.

Please let me know if you require any further information or please contact me on the details below should you want to discuss further.

Regards

**Andy Jones** 

Senior Infrastructure Engineer

#### **COOPERS**

Park House, Sandpiper Court, Chester Business Park, Chester, CH4 9QU

**2**: (01244) 684910 **2**: Direct Dial No. (01244) 684933

7: (01244) 684911

⊠: ajones@coopers.co.uk

Web: http://www.coopers.co.uk

Dwr Cymru Welsh Water is firmly committed to water conservation and promoting water efficiency. Please log on to our website www.dwrcymru.com/waterefficiency to find out how you can become water wise. Mae Dwr Cymru Welsh Water wedi ymrwymo i warchod adnoddau dwr a hyrwyddo defnydd dwr effeithiol. Mae cyngor i' ch helpu i ddefnyddio dwr yn ddoeth yn www.dwrcymru.com/waterefficiency

\*

# **Andy Jones**

**From:** Data Distribution <datadistribution@cyfoethnaturiolcymru.gov.uk>

**Sent:** 08 June 2022 09:44

To: Andy Jones

**Subject:** RE: FCA Historical Flood Information

Attachments: Briefing Note Flood Products - External.pdf; Flood Products FAQs V3.pdf; NRW Flood Product

Descriptions.pdf

Dear Mr Jones,

Thank you for your email concerning the above.

For flood history, please see this link - <u>Lle - Recorded Flood Extents (gov.wales)</u>

If you require flood data, please see the attached and then let us know.

We look forward to hearing from you.

Yn gywir / Yours sincerely,

#### Michelle Lewis

Cyfoeth Naturiol Cymru / Natural Resources Wales

Ffôn/ Phone: 03000 653577 Symudol / Mobile: 07917243096

Office Location Llys Afon, Hwlffordd / Office Location River Court, Haverfordwest Cwsmer, Cyfarthrebu a Masnach - Customer, Communications and Commercial

Oriau gwaith arferol/Normal working hours – Mon-Fri, 9 to 5

Yn falch o arwain y ffordd at ddyfodol gwell i Gymru trwy reoli'r amgylchedd ac adnoddau naturiol yn gynaliadwy.

Proud to be leading the way to a better future for Wales by managing the environment and natural resources sustainably.

cyfoethnaturiol.cymru / naturalresources.wales

Twitter | Facebook | LinkedIn | Instagram

From: Andy Jones <ajones@coopers.co.uk>

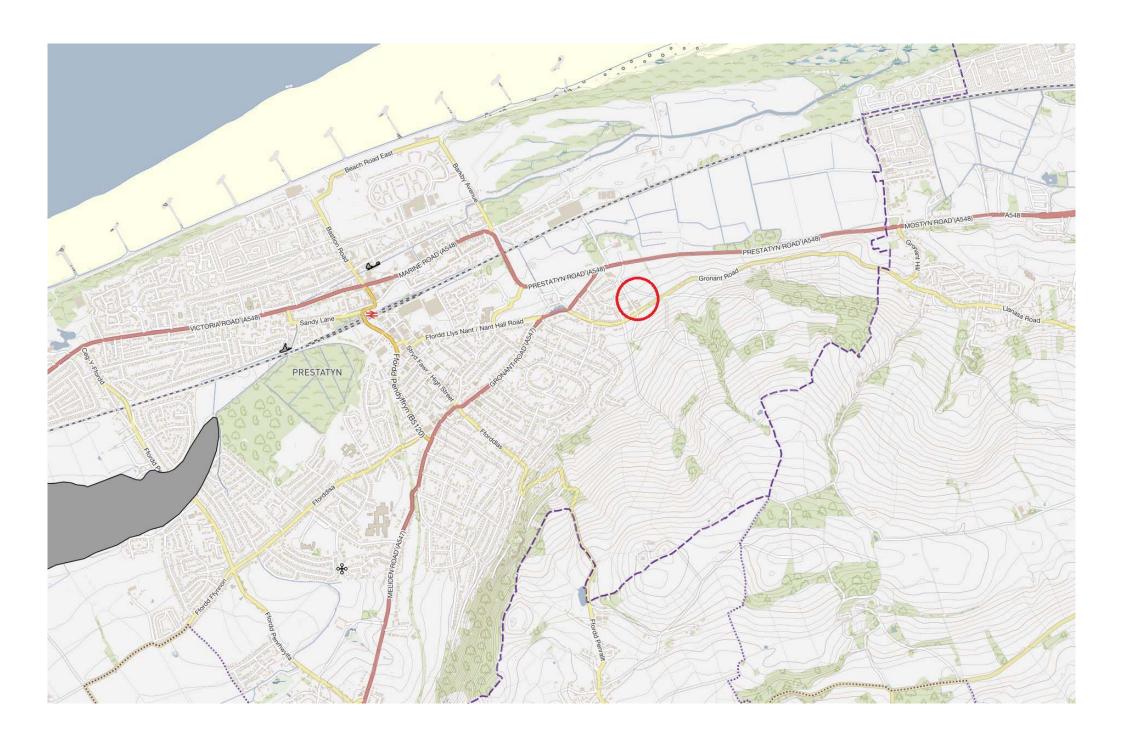
**Sent:** 07 June 2022 16:18

To: Data Distribution <datadistribution@cyfoethnaturiolcymru.gov.uk>

Subject: Fw: FCA Historical Flood Information

8007 Midnant Farm, Gronant Road, Prestatyn SJ077831 X=307750 , Y=383100

**FCA Historical Flood Information** 



### **Andy Jones**

**From:** Andy Jones **Sent:** 07 June 2022 16:16

To:yourvoice@denbighshire.gov.ukSubject:FCA Historical Flood InformationAttachments:Site Layout - Prestatyn - Rev A.pdf

8007 Midnant Farm, Gronant Road, Prestatyn SJ077831 X=307750 , Y=383100

### **FCA Historical Flood Information**

To whom it may concern

We are undertaking a Flood Consequences Assessment and Drainage Strategy for the above site (see attached Site Location Plan) and request any information you may have in relation to historical flooding or any information you may consider relevant to assist with the production of the FCA report.

Please let me know if you require any further information or please contact me on the details below should you want to discuss further.

### Regards

Andy Jones Senior Infrastructure Engineer

### **COOPERS**

Park House, Sandpiper Court, Chester Business Park, Chester, CH4 9QU

**2**: (01244) 684910 **2**: Direct Dial No. (01244) 684933

7: (01244) 684911

⊠: ajones@coopers.co.ukWeb: <a href="http://www.coopers.co.uk">http://www.coopers.co.uk</a>

# Flood Consequences Assessment and Drainage Strategy Midnant Farm, Gronant Road, Prestatyn, Denbighshire

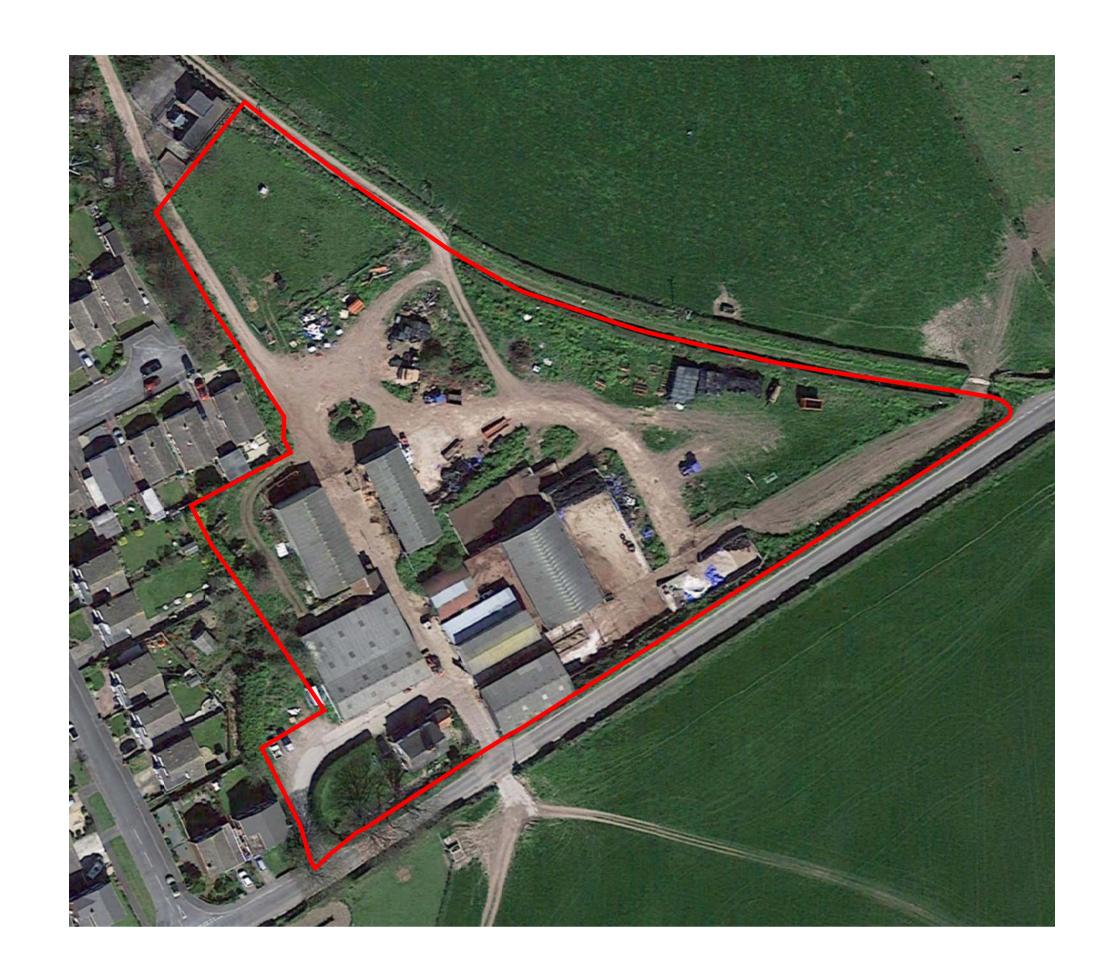
# Appendix 6

# **Drawings**

Drawing No.	Revision	<u>Title</u>
8007 / SK01	-	Existing Impermeable Areas
8007 / SK02	-	SW Opion 1 - Infiltration
8007 / SK03	-	Drainage Strategy
8007 / SK04	-	Highway Longsections

Our Ref: 8007\_FCA 25 November 2022





Legend

Site Area

2000sq.m. Roof Area

Existing Surface Water Flow Rates

Site Area = 1.45ha

Existing Brownfield Run-off Rates (Based on 2,000sq.m. existing roof area)

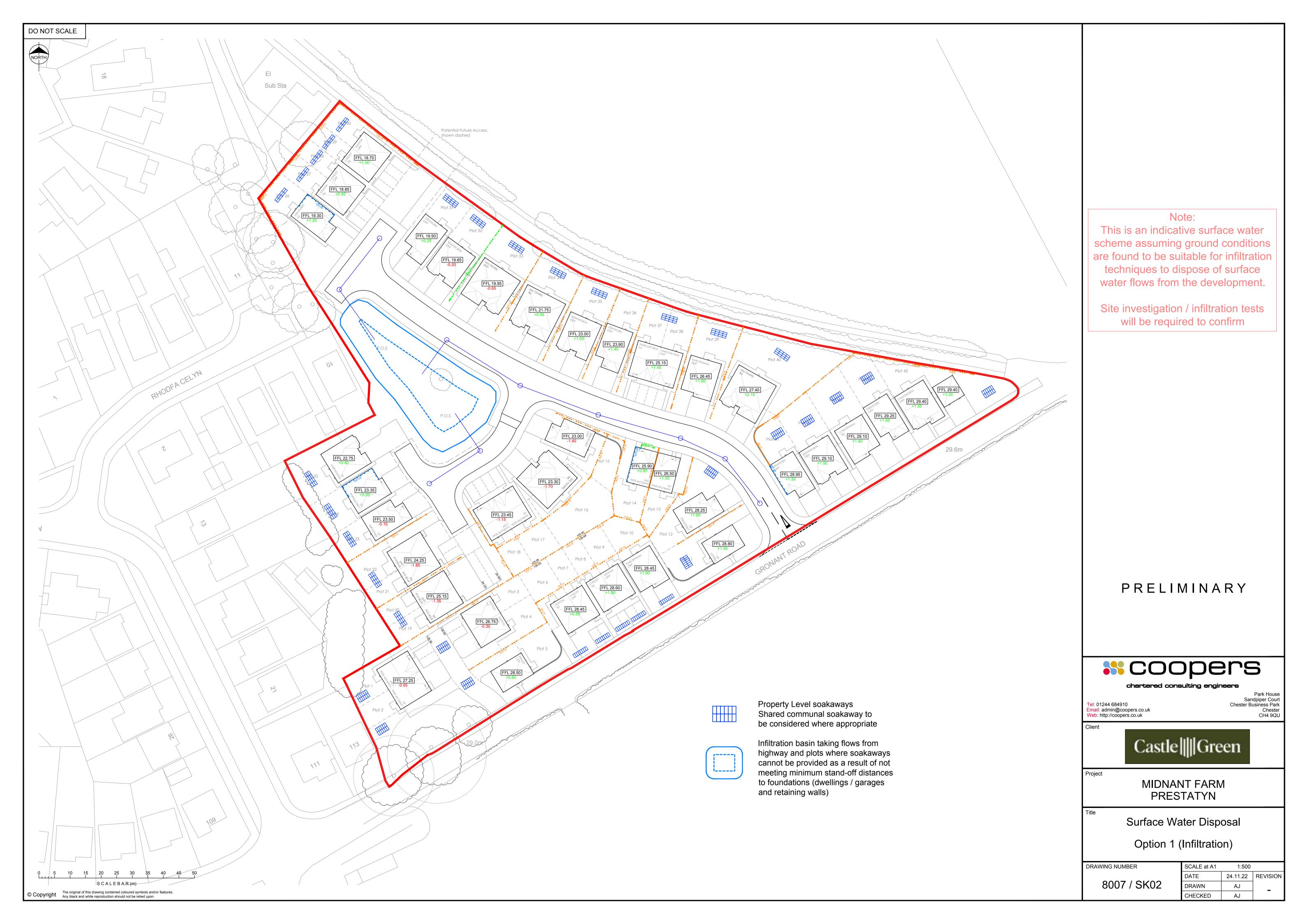
1yr = 13.98 l/s (25.8 mm/hr rainfall) 30yr = 34.20 l/s (63.1 mm/hr rainfall) 100yr = 44.12 l/s (81.4 mm/hr rainfall)

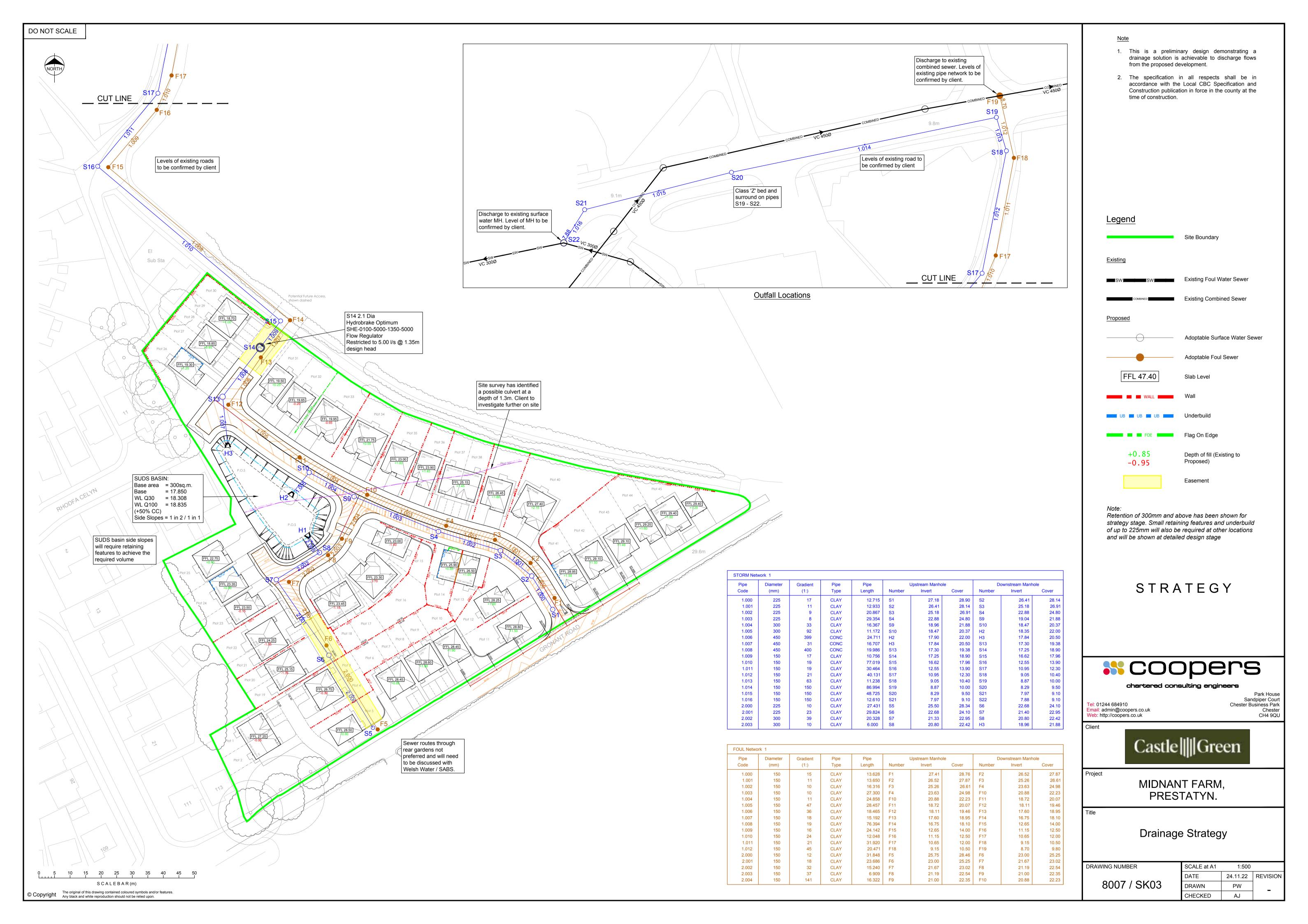
- First Issue
Rev. Date Revision \*\*coopers chartered consulting engineers Park House
684910 Sandpiper Court
Chester Business Park
Ain@coopers.co.uk Chester
CH4 9QU Tel: 01244 684910 Email: admin@coopers.co.uk Web: http://coopers.co.uk Client Midnant Farm

Prestatyn

Existing Impermeable Areas

SCALE at A1 1:500 DATE NOV 22 DRAWN CHECKED REVISION DRAWING NUMBER 8007 / SK01





DO NOT SCALE 30.000 25.000 Dia 450 IL 17.300 ROAD1 CHAINAGE EXISTING GROUND LEVEL ALIGNMENT LEVEL G= -5.000% L= 20.000 KF= 2.42418 G= -1.750% VERTICAL ALIGNMENT KF= -4.0 L= 20.000 1: -57.2 1: -10.0 HORIZONTAL ALIGNMENT R= 30.000 R= 200.000 STORMWATER COVER LEVEL Pipe 1.004 Dia 300 Circular CLAY 1 in 33 Pipe 1.001 & S Pipe 1.002 Dia 225 Pipe 1.003 Dia 225 Dia 225 CLAY 1 in 17 % Circular CLAY % Circular CLAY 1 in 9 Circular CLAY 1 in 8 STORMWATER INVERT 1 in 11 12.933 20.867 FOULWATER COVER LEVEL Pipe 1.002 Dia 150 Circular CLAY Pipe 1.003 Dia 150 Circular CLAY 1 in 10 Pipe 1.004 Dia 150 Circular CLAY 1 in 11 Pipe 1.005 Dia 150 Circular CLAY 1 in 47 20. N Circular CLAY N Circular CLAY N Circular 11 FOULWATER INVERT 1 in 10 27.300 24.858

# Notes

6 8 8 8

G= 2.859%

1: 35.0

Pipe 2.002 Dia 300 Circular CLAY

20.328

Pipe & & Pipe 2.002 2.003 - Dia 150

Pipe 2.004
Dia 150
Circular CLAY
1 in 141
CLAY
1 in 37
Dia 150
CLAY
1 in 37
Dia 150
CLAY
1 in 37
The property of the property

1 in 37 6.909

15.000

Pipe 2.003 Dia 300

Circular CLAY

25.000 **—** 

20.000 -

ROAD2

CHAINAGE

EXISTING GROUND LEVEL

ALIGNMENT LEVEL

VERTICAL ALIGNMENT

HORIZONTAL ALIGNMENT

STORMWATER COVER LEVEL

STORMWATER INVERT

FOULWATER COVER LEVEL

FOULWATER INVERT

- 1. All dimensions are in millimetres unless otherwise shown.
- 2. All adoptable drainage shall be constructed in accordance with 'Sewers for Adoption' 7th Edition, Welsh Ministers Standards and Welsh Water Details and Guidelines.
- 3. All private drainage works are to comply fully with Part H of the Building
- 4. All existing invert levels to be checked by the contractor at the start of works and any other discrepancies notified to the Engineer prior to commencing works. All levels are based on topographical survey information provided by others.
- 5. It is the responsibility of the Contractor to verify all information given with regards to existing services and drainage connections etc. prior to commencing the works.
- 6. The Contractor shall adhere to the CDM Regulations at all times.
- 7. Only trained personnel shall be permitted to enter confined spaces.
- 8. All materials to bear the relevant B.S. Kitemark and comply fully with the Sewers for Adoption 7th Specification. All concrete & concrete products must use Sulphate resistant cement (unless the site investigation report proves that sulphate attack from soils and groundwater will not occur to withstand a class 3 condition).
- 9. All opening notices etc. as required under Highways Acts etc. are to be obtained prior to commencement of works. All works are to be inspected by L.A., NHBC or Welsh Water as applicable.
- 10. Trench backfill in highways to within 1m of highway shall, as directed by the Highway Authority be a suitable granular material all in accordance with Sewers for Adoption.
- 11. Cover loadings shall be as detailed on the Manhole Schedule.
- 12. Drain trenches should not be excavated lower than the foundations of any building nearby unless either: a) The trench is within 1m of the foundation, the trench is filled with
  - concrete up to the lowest level of the foundation, or b) Where the trench is further than 1m from the building, the trench is filled with concrete to a level bellow the lowest level for the building equal to the distance from the build, less 150mm.
- 13. All SVP and RWP connections are indicative and shall be confirmed by the client.
- 14. Pipe gradients out of the building to connecting manhole to be laid at 1:40 in accordance with Building Regulations, Part H, Table 6.
- 15. Where pipe sizes are not indicated: 100Ø to be used for foul 100Ø to be used for surface water unless stated otherwise.
- 16. Minimum surface water gradients shall be: 100Ø laid at 1:100 with the exception of the first connection which shall be minimum 1:60 150Ø laid at 1:150
- 17. Minimum 100Ø foul drainage gradient to be 1:80 with the exception of the first connection which shall be minimum 1:40.
- 18. Manhole covers to be marked FWS or SW as appropriate.
- 19. All manhole covers and frames shall comply with BS EN124. All adoptable manholes and chambers shall comply with Sewers for Adoption 7th Edition. Covers in roads to be grade D400 and be 150mm deep. Manhole covers in car parking areas and drives to be grade B125 and covers in landscaping areas to be grade A15. All to be sized in accordance with Building Regulations Part H, Tables 11 & 12. "In-fill" type covers should not be used. Where a cover is located in an area of block paving, the bottom of the frame should be 150mm deep.
- 20. Precast concrete rings to be reinforced.
- 21. Backdrops in private manholes / inspection chambers to be internal
- 22. Private drains laid under adopted / private roads to be Class S granular bed and surround with a minimum of 1.2m cover, where this cannot be achieved a Class Z concrete bed and surround shall be provided.
- 23. Private drains located under landscape areas or driveways / car parking bays to be Class B granular bed and surround with a minimum 0.6m cover, where this cannot be achieved a Class Z concrete bed and surround shall be provided.
- 24. Pipes have not been designed to accommodate construction traffic loading. The contractor is responsible for providing adequate protection to the pipes during construction.
- 25. Slab levels shall not be varied without reference to the Engineer for guidance.
- 26. The developer must self-vet and certify that the design criteria, material standards and workmanship specifications for the proposed adoptable sewers are in accordance with those set out in "Sewers for Adoption" 7th Edition (SFA 7th), The Welsh Ministers Standards and the requirement of DCWW as the Statutory Sewerage Undertaker.
- 27. Subject to a Section 104 Adoption Agreement being complete, a Section 106 application to connect must be made to DCWW the developer shall be given 21 days notice prior to connection. The works may only be undertaken by an SSIP Health & Safety approved contractor.

Class 'S' bed and surround to all pipes unless stated otherwise. Class 'Z' bed and surround to pipes where cover < 1.2m as indicated on longsections.

All connections are soffit to soffit

STRATEGY



el: 01244 684910 mail: admin@coopers.co.uk

/eb: http://coopers.co.uk

Sandpiper Court Chester Business Park CH4 9QU

Castle||||Green

Project

Client

MIDNANT FARM, PRESTATYN.

Highway Longsections

DRAWING NUMBER SCALE at A1 1:500 H, 1:100 V 23.11.22 | REVISION DATE 8007 / SK04 DRAWN PW CHECKED AJ

# Flood Consequences Assessment and Drainage Strategy Midnant Farm, Gronant Road, Prestatyn, Denbighshire

# Appendix 7

# **MicroDrainage Calculation**

Source Control Greenfield Run-off Calculation

Preliminary Surface Water Design

Our Ref: 8007\_FCA 25 November 2022

# Print

# Close Report



# Greenfield runoff rate estimation for sites

www.uksuds.com | Greenfield runoff tool

Calculated by:	Andv	Jones					Site Details	
-							Latitude:	53.33681° N
Site name:	Midna	ant Farm					Longitudo	0.00040074
Site location:	Presta	atyn					Longitude:	3.38619° W
in line with Environme	ent Agenc ne SuDS M formation	y guidance Manual C7 on greenfi	e "Rainfall r '53 (Ciria, 2 eld runoff r	unoff manage 1015) and the	ement for de non-statutor	ry standards for SuDS	Reference:  Date:	3274103680 Jun 14 2022 16:54
Runoff estimati	on app	roach	IH124					
Site characteris	stics					Notes		
Total site area (ha	<b>):</b> 1.45					(1) In O	0.1/0/bo2	
Methodology						(1) Is Q <sub>BAR</sub> < 2.	.0 1/5/11a?	
Q <sub>BAR</sub> estimation r	nethod:	Calcu	ulate from	SPR and S	SAAR	When Q <sub>BAR</sub> is	< 2.0 l/s/ha then	limiting discharge rates are set
SPR estimation m	nethod:	Calcu	ulate from	SOIL type		at 2.0 l/s/ha.		
Soil characteris	R estimation method: Calculate from SOIL ty  il characteristics Default Edited							
SOIL type:		2		1		(2) Are flow rat	tes < 5.0 l/s?	
HOST class:		N/A	1	V/A		\\		: O 1/2
SPR/SPRHOST:		0.3	(	).47				i.0 l/s consent for discharge is from vegetation and other
Hydrological ch	naracte	ristics	Defa	ult	Edited			sent flow rates may be set essed by using appropriate
SAAR (mm):			718	7	18	drainage elem	<del>-</del>	based by daing appropriate
Hydrological region	n:		9	9		(2) to SDD/SDE	DUOCT ~ 0 22	
Growth curve fac	tor 1 yea	ar:	0.88	0	.88	(3) Is SPR/SPF	KHUSI ≤ 0.3?	
Growth curve fac	tor 30 ye	ears:	1.78	1	.78			w enough the use of
Growth curve fac	tor 100 y	/ears:	2.18	2	.18	11	avoid discharge c lisposal of surface	offsite would normally be water runoff.
Growth curve fac	tor 200 y	/ears:	2.46	2	.46			

Greenfield runoff rates	Default	Edited
Q <sub>BAR</sub> (I/s):	2.72	7.21
1 in 1 year (l/s):	2.4	6.35
1 in 30 years (l/s):	4.85	12.84
1 in 100 year (l/s):	5.93	15.72
1 in 200 years (l/s):	6.7	17.74

This report was produced using the greenfield runoff tool developed by HR Wallingford and available at www.uksuds.com. The use of this tool is subject to the UK SuDS terms and conditions and licence agreement, which can both be found at www.uksuds.com/terms-and-conditions.htm. The outputs from this tool are estimates of greenfield runoff rates. The use of these results is the responsibility of the users of this tool. No liability will be accepted by HR Wallingford, the Environment Agency, CEH, Hydrosolutions or any other organisation for the use of this data in the design or operational characteristics of any drainage scheme.

Coopers		Page 1
Park House	MIDNANT FARM, PRESATYN	
Sandpiper Court	Surface Water Design	
Chester CH4 9QU	PRELIMINARY	Mirro
Date 23/11/2022	Designed by Coopers	Drainage
File 8007 N1 SW01.MDX	Checked by AJ	Dialilade
Micro Drainage	Network 2020.1.3	,

#### STORM SEWER DESIGN by the Modified Rational Method

#### Design Criteria for 8007 SW01.SWS

Pipe Sizes 8007 SW01 Manhole Sizes 8007 SW01

FSR Rainfall Model - England and Wales

Return Period (years) 100

M5-60 (mm) 17.200

Ratio R 0.372

Maximum Rainfall (mm/hr) 50

Maximum Time of Concentration (mins) 30

Foul Sewage (1/s/ha) 0.000

Win Vel for Auto Design only (m/s) 0.75

Volumetric Runoff Coeff. 0.750

Model - England and Wales

PIMP (%) 100

Add Flow / Climate Change (%) 0

Minimum Backdrop Height (m) 1.500

Maximum Backdrop Height (m) 1.500

Min Design Depth for Optimisation (m) 1.200

Min Vel for Auto Design only (m/s) 0.75

Win Slope for Optimisation (1:X) 400

Designed with Level Soffits

#### Network Design Table for 8007 SW01.SWS

« - Indicates pipe capacity < flow

PN	Length	Fall	Slope	I.Area	T.E.	Base		k	HYD	DIA	Section Type	Auto
	(m)	(m)	(1:X)	(ha)	(mins)	Flow	(1/s)	(mm)	SECT	(mm)		Design
1.000	12.715	0.763	16.7	0.085	5.00		0.0	0.600	0	225	Pipe/Conduit	<del>8</del>
1.001	12.933	1.231	10.5	0.024	0.00		0.0	0.600	0	225	Pipe/Conduit	ă
1.002	20.867	2.307	9.0	0.014	0.00		0.0	0.600	0	225	Pipe/Conduit	ð
1.003	29.355	3.840	7.6	0.087	0.00		0.0	0.600	0	225	Pipe/Conduit	ē
2.000	27.431	2.825	9.7	0.102	5.00		0.0	0.600	0	225	Pipe/Conduit	<del>0</del>
2.001	29.824	1.274	23.4	0.000	0.00		0.0	0.600	0	225	Pipe/Conduit	<del>**</del>
2.002	20.328	0.526	38.6	0.098	0.00		0.0	0.600	0	300	Pipe/Conduit	ð
2.003	17.721	1.839	9.6	0.060	0.00		0.0	0.600	0	300	Pipe/Conduit	ð
1.004	16.367	0.489	33.5	0.085	0.00		0.0	0.600	0	300	Pipe/Conduit	<u> </u>
1.005	11.172	0.122	91.6	0.056	0.00		0.0	0.600	0	300	Pipe/Conduit	Ð
1.006	24.711	0.062	398.6	0.000	0.00		0.0	0.600	0	450	Pipe/Conduit	<del>8</del>
1.007	16.707	0.538	31.1	0.000	0.00		0.0	0.600	0	450	Pipe/Conduit	<del>**</del>

#### Network Results Table

PN	Rain (mm/hr)	T.C.	US/IL (m)	Σ I.Area (ha)	Σ Base Flow (1/s)	Foul (1/s)	Add Flow (1/s)	Vel (m/s)	Cap (1/s)	Flow (1/s)
1.000	50.00	5.07	27.177	0.085	0.0	0.0	0.0	3.22	128.1	11.5
1.001	50.00	5.12	26.414	0.109	0.0	0.0	0.0	4.06	161.4	14.8
1.002	50.00	5.20	25.183	0.123	0.0	0.0	0.0	4.38	174.0	16.7
1.003	50.00	5.30	22.876	0.210	0.0	0.0	0.0	4.76	189.4	28.4
2.000	50.00	5.11	25.500	0.102	0.0	0.0	0.0	4.22	168.0	13.8
2.001	50.00	5.29	22.675	0.102	0.0	0.0	0.0	2.72	108.0	13.8
2.002	50.00	5.42	21.326	0.200	0.0	0.0	0.0	2.54	179.3	27.1
2.003	50.00	5.48	20.800	0.260	0.0	0.0	0.0	5.09	360.0	35.2
1.004	50.00	5.58	18.961	0.555	0.0	0.0	0.0	2.73	192.8	75.2
1.005	50.00	5.70	18.472	0.611	0.0	0.0	0.0	1.64	116.2	82.7
1.006	50.00	6.10	17.900	0.611	0.0	0.0	0.0	1.01	161.0	82.7
1.007	50.00	6.18	17.838	0.611	0.0	0.0	0.0	3.66	581.9	82.7

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Coopers		Page 2
Park House	MIDNANT FARM, PRESATYN	
Sandpiper Court	Surface Water Design	
Chester CH4 9QU	PRELIMINARY	Micro
Date 23/11/2022	Designed by Coopers	Drainage
File 8007 N1 SW01.MDX	Checked by AJ	Diamage
Micro Drainage	Network 2020.1.3	,

# Network Design Table for 8007 SW01.SWS

PN	Length (m)	Fall (m)	Slope (1:X)	I.Area (ha)	T.E. (mins)	Ba Flow		k (mm)	HYD SECT	DIA (mm)	Section Type	Auto Design
1.008	19.986	0.050	399.7	0.061	0.00		0.0	0.600	0	450	Pipe/Conduit	<del> </del>
1.009	10.756	0.635	16.9	0.000	0.00		0.0	0.600	0	150	Pipe/Conduit	ĕ
1.010	77.019	4.065	18.9	0.000	0.00		0.0	0.600	0	150	Pipe/Conduit	ē
1.011	30.464	1.600	19.0	0.000	0.00		0.0	0.600	0	150	Pipe/Conduit	ē
1.012	40.131	1.900	21.1	0.000	0.00		0.0	0.600	0	150	Pipe/Conduit	ē
1.013	11.238	0.177	63.5	0.000	0.00		0.0	0.600	0	150	Pipe/Conduit	ē
1.014	86.994	0.580	150.0	0.000	0.00		0.0	0.600	0	150	Pipe/Conduit	ē
1.015	48.725	0.325	149.9	0.000	0.00		0.0	0.600	0	150	Pipe/Conduit	ē
1.016	12.610	0.084	150.1	0.000	0.00		0.0	0.600	0	150	Pipe/Conduit	ē

### Network Results Table

PN	Rain	T.C.	US/IL		Σ Base		Add Flow	Vel	Cap	Flow
	(mm/hr)	(mins)	(m)	(ha)	Flow (1/s)	(1/s)	(1/s)	(m/s)	(1/s)	(1/s)
1.008	50.00	6.51	17.300	0.672	0.0	0.0	0.0	1.01	160.7	91.0
1.009	50.00	6.58	17.250	0.672	0.0	0.0	0.0	2.46	43.5«	91.0
1.010	50.00	7.13	16.615	0.672	0.0	0.0	0.0	2.32	41.1«	91.0
1.011	50.00	7.35	12.550	0.672	0.0	0.0	0.0	2.32	41.0«	91.0
1.012	50.00	7.66	10.950	0.672	0.0	0.0	0.0	2.20	38.9≪	91.0
1.013	50.00	7.80	9.050	0.672	0.0	0.0	0.0	1.26	22.3«	91.0
1.014	50.00	9.58	8.873	0.672	0.0	0.0	0.0	0.82	14.5«	91.0
1.015	50.00	10.57	8.293	0.672	0.0	0.0	0.0	0.82	14.5«	91.0
1.016	50.00	10.83	7.968	0.672	0.0	0.0	0.0	0.82	14.5«	91.0

Coopers		Page 3
Park House	MIDNANT FARM, PRESATYN	
Sandpiper Court	Surface Water Design	
Chester CH4 9QU	PRELIMINARY	Micro
Date 23/11/2022	Designed by Coopers	Drainage
File 8007 N1 SW01.MDX	Checked by AJ	Dialilade
Micro Drainage	Network 2020.1.3	<u>'</u>

# Manhole Schedules for 8007 SW01.SWS

MH Name	MH CL (m)	MH Depth (m)	Conr	MH nection	MH Diam.,L*W (mm)	PN	Pipe Out Invert Level (m)	Diameter (mm)	PN	Pipes In Invert Level (m)	Diameter (mm)	Backdrop (mm)
1	28.902	1.725	Open	Manhole	1500	1.000	27.177	225				
2	28.139	1.725	Open	Manhole	1500	1.001	26.414	225	1.000	26.414	225	
3	26.908	1.725	Open	Manhole	1500	1.002	25.183	225	1.001	25.183	225	
4	24.801	1.925	Open	Manhole	1500	1.003	22.876	225	1.002	22.876	225	
5	28.339	2.839	Open	Manhole	1500	2.000	25.500	225				
6	24.100	1.425	Open	Manhole	1500	2.001	22.675	225	2.000	22.675	225	
7	22.951	1.625	Open	Manhole	1500	2.002	21.326	300	2.001	21.401	225	
8	22.425	1.625	Open	Manhole	1500	2.003	20.800	300	2.002	20.800	300	
9	21.884	2.923	Open	Manhole	1500	1.004	18.961	300	1.003	19.036	225	
									2.003	18.961	300	
10	20.372	1.900	Open	Manhole	1500	1.005	18.472	300	1.004	18.472	300	
11	22.000	4.100	Open	Manhole	1500	1.006	17.900	450	1.005	18.350	300	300
12	20.500	2.662	Open	Manhole	1500	1.007	17.838	450	1.006	17.838	450	
13	19.384	2.084	Open	Manhole	1800	1.008	17.300	450	1.007	17.300	450	
14	18.900	1.650	Open	Manhole	2100	1.009	17.250	150	1.008	17.250	450	
15	17.965	1.350	Open	Manhole	1350	1.010	16.615	150	1.009	16.615	150	
16	13.900	1.350	Open	Manhole	1350	1.011	12.550	150	1.010	12.550	150	
17	12.300	1.350	Open	Manhole	1350	1.012	10.950	150	1.011	10.950	150	
18	10.400	1.350	Open	Manhole	1350	1.013	9.050	150	1.012	9.050	150	
19	10.000	1.127	Open	Manhole	1350	1.014	8.873	150	1.013	8.873	150	
20	9.500	1.207	Open	Manhole	1350	1.015	8.293	150	1.014	8.293	150	
21	9.100	1.132	Open	Manhole	1350	1.016	7.968	150	1.015	7.968	150	
22	9.100	1.216	Open	Manhole	0		OUTFALL		1.016	7.884	150	

MH Iame	Manhole Easting (m)	Manhole Northing (m)	Intersection Easting (m)	Intersection Northing (m)	Manhole Access	Layout (North)
1	307824.048	383095.126	307824.048	383095.126	Required	>
2	307817.283	383105.892	307817.283	383105.892	Required	1
3	307807.245	383114.046	307807.245	383114.046	Required	
4	307787.305	383120.196	307787.305	383120.196	Required	-
5	307767.298	383057.354	307767.298	383057.354	Required	>
6	307752.648	383080.546	307752.648	383080.546	Required	1

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Coopers		Page 4
Park House	MIDNANT FARM, PRESATYN	
Sandpiper Court	Surface Water Design	
Chester CH4 9QU	PRELIMINARY	Micco
Date 23/11/2022	Designed by Coopers	Drainage
File 8007 N1 SW01.MDX	Checked by AJ	Dialilade
Micro Drainage	Network 2020.1.3	'

# Manhole Schedules for 8007 SW01.SWS

MH Name	Manhole Easting (m)	Manhole Northing (m)	Intersection Easting (m)	Intersection Northing (m)	Manhole Access	Layout (North)
7	307735.177	383104.716	307735.177	383104.716	Required	
8	307752.401	383115.511	307752.401	383115.511	Required	
9	307760.185	383131.431	307760.185	383131.431	Required	9.
10	307745.769	383139.180	307745.769	383139.180	Required	<b>9.</b>
11	307738.735	383130.501	307738.735	383130.501	Required	
12	307720.300	383146.957	307720.300	383146.957	Required	
13	307717.887	383163.488	307717.887	383163.488	Required	
14	307730.031	383179.362	307730.031	383179.362	Required	
15	307736.566	383187.904	307736.566	383187.904	Required	
16	307677.728	383237.603	307677.728	383237.603	Required	
17	307697.110	383261.106	307697.110	383261.106	Required	
18	307704.917	383300.470	307704.917	383300.470	Required	1
19	307701.702	383311.238	307701.702	383311.238	Required	1
20	307616.522	383293.564	307616.522	383293.564	Required	1
21	307569.301	383281.551	307569.301	383281.551	Required	9
22	307562.595	383270.873			No Entry	

Coopers		Page 5
Park House	MIDNANT FARM, PRESATYN	
Sandpiper Court	Surface Water Design	
Chester CH4 9QU	PRELIMINARY	Micro
Date 23/11/2022	Designed by Coopers	Drainage
File 8007 N1 SW01.MDX	Checked by AJ	pramade
Micro Drainage	Network 2020.1.3	

# PIPELINE SCHEDULES for 8007 SW01.SWS

# <u>Upstream Manhole</u>

PN	Hyd Sect	Diam	MH Name	C.Level (m)	I.Level	D.Depth (m)	MH Connection	MH DIAM., L*W (mm)
	sect	(111111)	Name	(111)	(111)	(111)	Connection	(11411)
1.000	0	225	1	28.902	27.177	1.500	Open Manhole	1500
1.001	0	225	2	28.139	26.414	1.500	Open Manhole	1500
1.002	0	225	3	26.908	25.183	1.500	Open Manhole	1500
1.003	0	225	4	24.801	22.876	1.700	Open Manhole	1500
2.000	0	225	5	28.339	25.500	2.614	Open Manhole	1500
2.001	0	225	6	24.100	22.675	1.200	Open Manhole	1500
2.002	0	300	7	22.951	21.326	1.325	Open Manhole	1500
2.003	0	300	8	22.425	20.800	1.325	Open Manhole	1500
1.004	0	300	9	21.884	18.961	2.623	Open Manhole	1500
1.005	0	300	10	20.372	18.472	1.600	Open Manhole	1500
1.006	0	450	11	22.000	17.900	3.650	Open Manhole	1500
1.007	0	450	12	20.500	17.838	2.212	Open Manhole	1500
1.008	0	450	13	19.384	17.300	1.634	Open Manhole	1800
1.009	0	150	14	18.900	17.250	1.500	Open Manhole	2100
1.010	0	150	15	17.965	16.615	1.200	Open Manhole	1350
1.011	0	150	16	13.900	12.550	1.200	Open Manhole	1350
1.012	0	150	17	12.300	10.950	1.200	Open Manhole	1350
1.013	0	150	18	10.400	9.050	1.200	Open Manhole	1350
1.014	0	150	19	10.000	8.873	0.977	Open Manhole	1350
1.015	0	150	20	9.500	8.293	1.057	Open Manhole	1350
1.016	0	150	21	9.100	7.968	0.982	Open Manhole	1350

# Downstream Manhole

PN	Length	Slope	MH	C.Level	I.Level	D.Depth	MH	MH DIAM., L*W
	(m)	(1:X)	Name	(m)	(m)	(m)	Connection	(mm)
1.000	12.715	16.7	2	28.139	26.414	1.500	Open Manhole	1500
1.001	12.933	10.5	3	26.908	25.183		Open Manhole	1500
1.002	20.867	9.0	4	24.801	22.876	1.700	Open Manhole	1500
1.003	29.355	7.6	9	21.884	19.036	2.623	Open Manhole	1500
2.000	27.431	9.7	6	24.100	22.675	1.200	Open Manhole	1500
2.001	29.824	23.4	7	22.951	21.401	1.325	Open Manhole	1500
2.002	20.328	38.6	8	22.425	20.800	1.325	Open Manhole	1500
2.003	17.721	9.6	9	21.884	18.961	2.623	Open Manhole	1500
1.004	16.367	33.5	10	20.372	18.472	1.600	Open Manhole	1500
1.005	11.172	91.6	11	22.000	18.350	3.350	Open Manhole	1500
1.006	24.711	398.6	12	20.500	17.838	2.212	Open Manhole	1500
1.007	16.707	31.1	13	19.384	17.300	1.634	Open Manhole	1800
1.008	19.986	399.7	14	18.900	17.250	1.200	Open Manhole	2100
1.009	10.756	16.9	15	17.965	16.615	1.200	Open Manhole	1350
1.010	77.019	18.9	16	13.900	12.550	1.200	Open Manhole	1350
1.011	30.464	19.0	17	12.300	10.950	1.200	Open Manhole	1350
1.012	40.131	21.1	18	10.400	9.050	1.200	Open Manhole	1350
1.013	11.238	63.5	19	10.000	8.873	0.977	Open Manhole	1350
1.014	86.994	150.0	20	9.500	8.293	1.057	Open Manhole	1350
1.015	48.725	149.9	21	9.100	7.968	0.982	Open Manhole	1350
1.016	12.610	150.1	22	9.100	7.884	1.066	Open Manhole	0

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Coopers				
Park House	MIDNANT FARM, PRESATYN			
Sandpiper Court	Surface Water Design			
Chester CH4 9QU	PRELIMINARY	Micro		
Date 23/11/2022	Designed by Coopers	Drainage		
File 8007 N1 SW01.MDX	Checked by AJ	niamade		
Micro Drainage	Network 2020.1.3			

#### Free Flowing Outfall Details for 8007 SW01.SWS

Outfall Outfall C. Level I. Level Min D,L W
Pipe Number Name (m) (m) I. Level (mm) (mm)
(m)

1.016 22 9.100 7.884 0.000 0

#### Simulation Criteria for 8007 SW01.SWS

Volumetric Runoff Coeff 0.750 Additional Flow - % of Total Flow 0.000
Areal Reduction Factor 1.000 MADD Factor \* 10m³/ha Storage 2.000
Hot Start (mins) 0 Inlet Coefficient 0.800
Hot Start Level (mm) 0 Flow per Person per Day (1/per/day) 0.000
Manhole Headloss Coeff (Global) 0.500 Run Time (mins) 60
Foul Sewage per hectare (1/s) 0.000 Output Interval (mins) 1

Number of Input Hydrographs 0 Number of Offline Controls 0 Number of Time/Area Diagrams 0 Number of Online Controls 1 Number of Storage Structures 1 Number of Real Time Controls 0

#### Synthetic Rainfall Details

Rainfall Model	FSR	Profile Type St	ummer
Return Period (years)	100	Cv (Summer)	0.750
Region	England and Wales	Cv (Winter)	0.840
M5-60 (mm)	17.200	Storm Duration (mins)	30
Ratio R	0.372		

Coopers					
Park House	MIDNANT FARM, PRESATYN				
Sandpiper Court	Surface Water Design				
Chester CH4 9QU	PRELIMINARY	Micro			
Date 23/11/2022	Designed by Coopers	Drainage			
File 8007 N1 SW01.MDX	Checked by AJ	niamade			
Micro Drainage	Network 2020.1.3				

#### Online Controls for 8007 SW01.SWS

### Hydro-Brake® Optimum Manhole: 14, DS/PN: 1.009, Volume (m³): 8.6

Unit Reference MD-SHE-0100-5000-1350-5000 Design Head (m) 1.350 Design Flow (1/s) 5.0 Flush-Flo™ Calculated Objective Minimise upstream storage Application Surface Sump Available Yes Diameter (mm) 100 17.250 Invert Level (m) Minimum Outlet Pipe Diameter (mm) 150 Suggested Manhole Diameter (mm) 1200

Control Points	Head (m)	Flow (1/s)	Control Points	Head (m)	Flow (1/s)
Design Point (Calculated)	1.350	5.0	Kick-Flo®	0.829	4.0
Flush-Flo™	0.400	5.0	Mean Flow over Head Range	-	4.4

The hydrological calculations have been based on the Head/Discharge relationship for the Hydro-Brake® Optimum as specified. Should another type of control device other than a Hydro-Brake Optimum® be utilised then these storage routing calculations will be invalidated

Depth (m) Fl	ow (1/s)	Depth (m) Fl	low (1/s)	Depth (m) Fl	ow (1/s)	Depth (m)	Flow $(1/s)$
0.100	3.3	1.200	4.7	3.000	7.3	7.000	10.9
0.200	4.6	1.400	5.1	3.500	7.8	7.500	11.2
0.300	4.9	1.600	5.4	4.000	8.3	8.000	11.6
0.400	5.0	1.800	5.7	4.500	8.8	8.500	11.9
0.500	5.0	2.000	6.0	5.000	9.2	9.000	12.2
0.600	4.8	2.200	6.3	5.500	9.7	9.500	12.6
0.800	4.2	2.400	6.5	6.000	10.1		
1.000	4.4	2.600	6.8	6.500	10.5		

Coopers		Page 8
Park House	MIDNANT FARM, PRESATYN	
Sandpiper Court	Surface Water Design	
Chester CH4 9QU	PRELIMINARY	Micco
Date 23/11/2022	Designed by Coopers	Drainage
File 8007 N1 SW01.MDX	Checked by AJ	Diali lade
Micro Drainage	Network 2020.1.3	

### Storage Structures for 8007 SW01.SWS

# Tank or Pond Manhole: 12, DS/PN: 1.007

Invert Level (m) 17.838

Depth (m) Area (m<sup>2</sup>) 0.000 300.0 0.400 375.0 0.800 450.0 1.200 530.0

# Volume Summary (Static)

Length Calculations based on Centre-Centre

				Storage	
Pipe			Pipe		Total
Number	Name	Volume (m³)	Volume (m³)	Volume (m³)	Volume (m³)
1.000	1	3.048	0.506	0.000	3.554
1.000	2	3.048	0.514	0.000	3.563
1.001	3	3.048	0.830	0.000	3.878
	4				4.569
1.003		3.402	1.167	0.000	
2.000	5	5.017	1.091	0.000	6.108
2.001	6	2.518	1.186	0.000	3.704
2.002	7	2.872	1.437	0.000	4.309
2.003	8	2.872	1.253	0.000	4.124
1.004	9	5.165	1.157	0.000	6.322
1.005	10	3.358	0.790	0.000	4.147
1.006	11	7.245	3.930	0.000	11.175
1.007	12	4.704	2.657	1270.136	1277.497
1.008	13	5.303	3.179	0.000	8.482
1.009	14	5.715	0.190	0.000	5.905
1.010	15	1.932	1.361	0.000	3.293
1.011	16	1.932	0.538	0.000	2.471
1.012	17	1.932	0.709	0.000	2.642
1.013	18	1.932	0.199	0.000	2.131
1.014	19	1.613	1.537	0.000	3.150
1.015	20	1.728	0.861	0.000	2.589
1.016	21	1.620	0.223	0.000	1.843
Total		70.006	25.313	1270.136	1365.455

Coopers		Page 9
Park House	MIDNANT FARM, PRESATYN	
Sandpiper Court	Surface Water Design	
Chester CH4 9QU	PRELIMINARY	Micro
Date 23/11/2022	Designed by Coopers	Drainage
File 8007 N1 SW01.MDX	Checked by AJ	niamade
Micro Drainage	Network 2020.1.3	

# $\frac{\text{1 year Return Period Summary of Critical Results by Maximum Level (Rank 1) for 8007}}{\text{SW01.SWS}}$

#### Simulation Criteria

Areal Reduction Factor 1.000 Additional Flow - % of Total Flow 0.000 Hot Start (mins) 0 MADD Factor \*  $10m^3$ /ha Storage 2.000 Hot Start Level (mm) 0 Inlet Coefficient 0.800 Manhole Headloss Coeff (Global) 0.500 Flow per Person per Day (1/per/day) 0.000 Foul Sewage per hectare (1/s) 0.000

Number of Input Hydrographs 0 Number of Offline Controls 0 Number of Time/Area Diagrams 0 Number of Online Controls 1 Number of Storage Structures 1 Number of Real Time Controls 0

#### Synthetic Rainfall Details

Rainfall Model FSR M5-60 (mm) 17.200 Cv (Summer) 0.750 Region England and Wales Ratio R 0.371 Cv (Winter) 0.840

Margin for Flood Risk Warning (mm) 300.0

Analysis Timestep 2.5 Second Increment (Extended)

DTS Status

OFF

DVD Status

ON

Inertia Status

Profile(s) Summer and Winter Duration(s) (mins) 15, 30, 60, 120, 180, 240, 360, 480, 600, 720, 960, 1440
Return Period(s) (years) 1, 30, 100
Climate Change (%) 0, 0, 50

											Water	Surcharged
	US/MH			Return	Climate	First	t (X)	First (Y)	First (Z)	Overflow	Level	Depth
PN	Name	S	torm	Period	Change	Surcl	narge	Flood	Overflow	Act.	(m)	(m)
1.000	1		Winter	1	+0%						27.222	-0.180
1.001	2	15	Winter	1	+0%						26.459	-0.180
1.002	3	15	Winter	1	+0%						25.228	-0.180
1.003	4	15	Winter	1	+0%						22.929	-0.172
2.000	5	15	Winter	1	+0%						25.541	-0.184
2.001	6	15	Winter	1	+0%						22.726	-0.174
2.002	7	15	Winter	1	+0%	100/15	Summer				21.400	-0.226
2.003	8	15	Winter	1	+0%	100/15	Summer				20.860	-0.240
1.004	9	15	Winter	1	+0%	30/15	Summer				19.084	-0.177
1.005	10	15	Winter	1	+0%	30/15	Summer				18.663	-0.109
1.006	11	15	Winter	1	+0%	30/15	Summer				18.116	-0.234
1.007	12	120	Winter	1	+0%	100/30	Summer				17.969	-0.319
1.008	13	120	Winter	1	+0%	1/15	Summer				17.976	0.226
1.009	14	120	Winter	1	+0%	1/15	Summer				17.976	0.576
1.010	15	1440	Summer	1	+0%						16.649	-0.116
1.011	16	1440	Summer	1	+0%						12.585	-0.115
1.012	17	1440	Summer	1	+0%						10.986	-0.114
1.013	18	1440	Summer	1	+0%						9.100	-0.100
1.014	19	1440	Summer	1	+0%						8.933	-0.090
1.015			Summer	1	+0%						8.354	-0.089
1.016	21	1440	Summer	1	+0%						8.031	-0.087
				_								

Coopers		Page 10
Park House	MIDNANT FARM, PRESATYN	
Sandpiper Court	Surface Water Design	
Chester CH4 9QU	PRELIMINARY	Mirro
Date 23/11/2022	Designed by Coopers	Drainage
File 8007 N1 SW01.MDX	Checked by AJ	Digitiarie.
Micro Drainage	Network 2020.1.3	

# $\frac{\text{1 year Return Period Summary of Critical Results by Maximum Level (Rank 1) for 8007}}{\text{SW01.SWS}}$

		Flooded			Half Drain	Pipe		
	US/MH	Volume	Flow /	Overflow	Time	Flow		Level
PN	Name	(m³)	Cap.	(1/s)	(mins)	(l/s)	Status	Exceeded
1 000	1	0 000	0 00			0 0	O.V.	
1.000	1	0.000	0.09			9.8	OK	
1.001	2	0.000	0.09			12.1	OK	
1.002	3	0.000	0.09			13.6	OK	
1.003	4	0.000	0.13			22.1	OK	
2.000	5	0.000	0.08			11.8	OK	
2.001	6	0.000	0.12			11.7	OK	
2.002	7	0.000	0.14			21.3	OK	
2.003	8	0.000	0.09			27.3	OK	
1.004	9	0.000	0.35			57.8	OK	
1.005	10	0.000	0.72			63.4	OK	
1.006	11	0.000	0.46			62.5	OK	
1.007	12	0.000	0.04			15.9	OK	
1.008	13	0.000	0.09			11.4	SURCHARGED	
1.009	14	0.000	0.12			4.9	SURCHARGED	
1.010	15	0.000	0.12			4.9	OK	
1.011	16	0.000	0.12			4.9	OK	
1.012	17	0.000	0.13			4.9	OK	
1.013	18	0.000	0.24			4.9	OK	
1.014	19	0.000	0.34			4.9	OK	
1.015	20	0.000	0.35			4.9	OK	
1.016	21	0.000	0.37			4.9	OK	

Coopers		Page 11
Park House	MIDNANT FARM, PRESATYN	
Sandpiper Court	Surface Water Design	
Chester CH4 9QU	PRELIMINARY	Mirro
Date 23/11/2022	Designed by Coopers	Drainage
File 8007 N1 SW01.MDX	Checked by AJ	Diamage
Micro Drainage	Network 2020.1.3	

# 30 year Return Period Summary of Critical Results by Maximum Level (Rank 1) for 8007 SW01.SWS

#### Simulation Criteria

Areal Reduction Factor 1.000 Additional Flow - % of Total Flow 0.000
Hot Start (mins) 0 MADD Factor \* 10m³/ha Storage 2.000
Hot Start Level (mm) 0 Inlet Coefficient 0.800
Manhole Headloss Coeff (Global) 0.500 Flow per Person per Day (1/per/day) 0.000
Foul Sewage per hectare (1/s) 0.000

Number of Input Hydrographs 0 Number of Offline Controls 0 Number of Time/Area Diagrams 0 Number of Online Controls 1 Number of Storage Structures 1 Number of Real Time Controls 0

#### Synthetic Rainfall Details

Rainfall Model FSR M5-60 (mm) 17.200 Cv (Summer) 0.750 Region England and Wales Ratio R 0.371 Cv (Winter) 0.840

Margin for Flood Risk Warning (mm) 300.0

Analysis Timestep 2.5 Second Increment (Extended)

DTS Status

OFF

DVD Status

ON

Inertia Status

Profile(s) Summer and Winter Duration(s) (mins) 15, 30, 60, 120, 180, 240, 360, 480, 600, 720, 960, 1440
Return Period(s) (years) 1, 30, 100
Climate Change (%) 0, 0, 50

											Water	Surcharged
	US/MH			Return	${\tt Climate}$	First	t (X)	First (Y)	First (Z)	Overflow	Level	Depth
PN	Name	S	torm	Period	Change	Surcl	narge	Flood	Overflow	Act.	(m)	(m)
1.000	1		Winter	30	+0%						27.248	-0.154
1.001	2	15	Winter	30	+0%						26.486	-0.153
1.002	3	15	Winter	30	+0%						25.256	-0.152
1.003	4	15	Winter	30	+0%						22.968	-0.133
2.000	5	15	Winter	30	+0%						25.566	-0.159
2.001	6	15	Winter	30	+0%						22.757	-0.143
2.002	7	15	Winter	30	+0%	100/15	Summer				21.454	-0.172
2.003	8	15	Winter	30	+0%	100/15	Summer				20.902	-0.198
1.004	9	15	Winter	30	+0%	30/15	Summer				19.607	0.346
1.005	10	15	Winter	30	+0%	30/15	Summer				19.129	0.357
1.006	11	15	Winter	30	+0%	30/15	Summer				18.369	0.019
1.007	12	240	Winter	30	+0%	100/30	Summer				18.274	-0.014
1.008	13	240	Winter	30	+0%	1/15	Summer				18.279	0.529
1.009	14	240	Winter	30	+0%	1/15	Summer				18.279	0.879
1.010	15	480	Summer	30	+0%						16.649	-0.116
1.011	16	720	Winter	30	+0%						12.585	-0.115
1.012	17	1440	Summer	30	+0%						10.986	-0.114
1.013	18	480	Summer	30	+0%						9.100	-0.100
1.014	19	720	Winter	30	+0%						8.933	-0.090
1.015	20	1440	Summer	30	+0%						8.354	-0.089
1.016	21	1440	Summer	30	+0%						8.031	-0.087

Coopers		Page 12
Park House	MIDNANT FARM, PRESATYN	
Sandpiper Court	Surface Water Design	
Chester CH4 9QU	PRELIMINARY	Micro
Date 23/11/2022	Designed by Coopers	Drainage
File 8007 N1 SW01.MDX	Checked by AJ	Diamage
Micro Drainage	Network 2020.1.3	

# $\frac{30~\text{year}}{\text{Return}}$ Period Summary of Critical Results by Maximum Level (Rank 1) for 8007 $\underline{\text{SW01.SWS}}$

		Flooded			Half Drain	Pipe		
	US/MH	Volume	Flow /	Overflow	Time	Flow		Level
PN	Name	(m³)	Cap.	(1/s)	(mins)	(1/s)	Status	Exceeded
1.000	1	0.000	0.22			24.1	OK	
1.001	2		0.22			31.2		
1.002	3		0.22			35.2		
1.002	4		0.35			61.3		
2.000	5		0.19			29.0		
2.001	6	0.000	0.28			28.6		
2.002	7		0.37			58.1		
2.003	8	0.000	0.25			76.0		
1.004	9		0.97				SURCHARGED	
1.005	10	0.000	2.00				SURCHARGED	
1.006	11	0.000	1.27				SURCHARGED	
1.007	12	0.000	0.03				OK	
1.008	13	0.000	0.06				SURCHARGED	
1.009	14	0.000	0.12				SURCHARGED	
1.010	15	0.000	0.12			4.9		
1.011	16	0.000	0.12			4.9		
1.012	17	0.000	0.13			4.9	OK	
1.013	18	0.000	0.24			4.9		
1.014	19	0.000	0.34			4.9	OK	
1.015	20	0.000	0.35			4.9	OK	
1.016	21	0.000	0.37			4.9	OK	

Coopers		Page 13
Park House	MIDNANT FARM, PRESATYN	
Sandpiper Court	Surface Water Design	
Chester CH4 9QU	PRELIMINARY	Mirro
Date 23/11/2022	Designed by Coopers	Drainage
File 8007 N1 SW01.MDX	Checked by AJ	Dialilade
Micro Drainage	Network 2020.1.3	'

# 100 year Return Period Summary of Critical Results by Maximum Level (Rank 1) for 8007 SW01.SWS

#### Simulation Criteria

Areal Reduction Factor 1.000 Additional Flow - % of Total Flow 0.000 Hot Start (mins) 0 MADD Factor \*  $10m^3$ /ha Storage 2.000 Hot Start Level (mm) 0 Inlet Coefficient 0.800 Manhole Headloss Coeff (Global) 0.500 Flow per Person per Day (1/per/day) 0.000 Foul Sewage per hectare (1/s) 0.000

Number of Input Hydrographs 0 Number of Offline Controls 0 Number of Time/Area Diagrams 0 Number of Online Controls 1 Number of Storage Structures 1 Number of Real Time Controls 0

#### Synthetic Rainfall Details

Rainfall Model FSR M5-60 (mm) 17.200 Cv (Summer) 0.750 Region England and Wales Ratio R 0.371 Cv (Winter) 0.840

Margin for Flood Risk Warning (mm) 300.0

Analysis Timestep 2.5 Second Increment (Extended)

DTS Status

OFF

DVD Status

ON

Inertia Status

													Water	Surcharged
	US/MH			Return	Climate	First	(X)	First	(Y)	First	(Z)	Overflow	Level	Depth
PN	Name	s	torm	Period	Change	Surch	narge	Floo	od	Overf	low	Act.	(m)	(m)
1.000	1	1 5	Winter	100									27.280	-0.122
	_				+50%									
1.001	2		Winter		+50%								26.518	-0.121
1.002	3		Winter	100	+50%								25.287	-0.121
1.003	4	15	Winter	100	+50%								23.023	-0.078
2.000	5	15	Winter	100	+50%								25.594	-0.131
2.001	6	15	Winter	100	+50%								22.796	-0.104
2.002	7	15	Winter	100	+50%	100/15	Summer						22.019	0.393
2.003	8	15	Winter	100	+50%	100/15	Summer						21.798	0.698
1.004	9	15	Winter	100	+50%	30/15	Summer						21.356	2.095
1.005	10	15	Winter	100	+50%	30/15	Summer						20.027	1.255
1.006	11	360	Winter	100	+50%	30/15	Summer						18.770	0.420
1.007	12	360	Winter	100	+50%	100/30	Summer						18.768	0.480
1.008	13	480	Winter	100	+50%	1/15	Summer						18.816	1.066
1.009	14	480	Winter	100	+50%	1/15	Summer						18.833	1.433
1.010	15	480	Winter	100	+50%								16.651	-0.114
1.011	16	480	Winter	100	+50%								12.586	-0.114
1.012	17	480	Winter	100	+50%								10.987	-0.113
1.013	18	360	Winter	100	+50%								9.102	-0.098
1.014	19	480	Winter	100	+50%								8.936	-0.087
1.015	20	480	Winter	100	+50%								8.356	-0.087
1.016	21	480	Winter	100	+50%								8.034	-0.084

Coopers	Page 14	
Park House	MIDNANT FARM, PRESATYN	
Sandpiper Court	Surface Water Design	
Chester CH4 9QU	PRELIMINARY	Micco
Date 23/11/2022	Designed by Coopers	Drainage
File 8007 N1 SW01.MDX	Checked by AJ	Diali lade
Micro Drainage	Network 2020.1.3	

# $\frac{\text{100 year Return Period Summary of Critical Results by Maximum Level (Rank 1) for 8007}}{\text{SW01.SWS}}$

		Flooded			Half Drain	Pipe		
	US/MH	Volume	Flow /	Overflow	Time	Flow		Level
PN	Name	(m³)	Cap.	(1/s)	(mins)	(1/s)	Status	Exceeded
1.000	1	0.000	0.42			46.7	OK	
1.000	2		0.42			60.3		
1.001	3		0.43			68.1		
1.002		0.000	0.43					
	4					118.4		
2.000	5		0.36			56.1		
2.001	6	0.000	0.55			55.5		
2.002	7		0.66				SURCHARGED	
2.003	8	0.000	0.44			134.7	SURCHARGED	
1.004	9	0.000	1.63			266.5	SURCHARGED	
1.005	10	0.000	3.35			293.1	SURCHARGED	
1.006	11	0.000	0.38			50.8	SURCHARGED	
1.007	12	0.000	0.06			23.7	SURCHARGED	
1.008	13	0.000	0.11			14.7	SURCHARGED	
1.009	14	0.000	0.14			5.3	FLOOD RISK	
1.010	15	0.000	0.13			5.3	OK	
1.011	16	0.000	0.13			5.3	OK	
1.012	17	0.000	0.14			5.3	OK	
1.013	18	0.000	0.26			5.3	OK	
1.014	19	0.000	0.37			5.3	OK	
1.015	20	0.000	0.37			5.3	OK	
1.016	21	0.000	0.40			5.3	OK	